Stress computations on perforated polygonal domains

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Abstract

A high order accurate and fast algorithm is constructed for 2D stress problems on multiply connected finite domains. The algorithm is based on a Fredholm integral equation of the second kind with non-singular operators. The unknown quantity is the limit of an analytic function. On polygonal domains there is a trade-off between stability and rate of convergence. A moderate amount of precomputation in higher precision arithmetic increases the stability in difficult situations. Results for a loaded single edge notched specimen perforated with 1170 holes are presented. The general usefulness of integral equation methods is discussed.
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1. Introduction

The problem of solving for stress in a loaded, linearly elastic body has received attention for almost a century. Early work dealt with formulations of the governing equations. Integral equations appeared as a natural tool both for analysis and for obtaining solutions. See the textbooks of Mikhlin [40], Muskhelishvili [41] and Sokolnikoff [48]. Naturally, only very simple setups can be solved by purely analytical methods.

Later work on stress problems has focused more on numerical techniques for particular applications. See Refs. [7,9,10,14,30,38,50,52] for recent 2D examples involving a variety of methods of which some are based on integral equations. Parallel to this development there has been a steady improvement of commercial general purpose finite element packages, such as ANSYS and ABAQUS. These codes can, among other things, solve for stress on multiply connected 2D domains. In fact, in Ref. [29] we showed that for simple setups, involving a few holes and cracks in finite domains, these commercial packages often perform better than many specialized codes. Similar observations made by others has lead to a belief in the solid mechanics community that the finite element method will be the only numerical

method in the future. Once computer processors become fast enough, and computer memories become large enough, the need for other numerical techniques will disappear.

In this paper we argue and demonstrate that integral equation methods are indeed extremely competitive, that is, when they apply and if they are implemented carefully. We ascribe their limited use in computational solid mechanics to insufficient work on good equations for problems of interest. The paper is organized as follows: Section 2 discusses advantages with and shortcomings of integral equation methods. Section 3 reviews a recently derived and very efficient equation for multiply connected 2D domains [29]. Sections 4 and 5 contain the new material. Section 4 gives details on the present discretization of the integral equation. Particular emphasis is given to the quadrature in corners. Our scheme for resolving the stress field in corners, originally developed in Ref. [28], makes use of the analytic properties of the unknown quantity, and should be seen as an alternative to the more general scheme of Cheng, Yarvin and Rokhlin [12,53]. New placement of discretization points increases the stability. Higher order quadrature increases the convergence rate, compared to the earlier implementation [28,29]. Section 5 gives several numerical examples. Among other things, we compute the notch stress intensity factor and the stress concentration factor for a loaded single edge notched specimen perforated with 1170 circular holes.

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2. Why are integral equation methods needed?

The chief advantage of integral equation methods over methods based on partial differential equations has to do with stability. With stability, in this context, we mean that the truncation error should decrease when the number of discretization points is increased while the rounding error should stay bounded. The rounding error is not bounded for methods that, upon discretization, lead to system matrices whose condition numbers diverge with the number of discretization points.

Since many problems in linear fracture mechanics are well conditioned [39], a good numerical method for these problems should result in discretized problems with low condition numbers. Algorithms based on partial differential equation formulations do not have this property. Differential operators are unbounded and when the discretization is refined, the rounding error increases. These undesirable effects could, in theory, be offset by preconditioners using techniques such as multigrid [24] or domain decomposition [4]. Still, for many realistic problems and geometries it is often difficult to achieve optimal performance [15]. The total work tends to grow faster than linearly with the number of discretization points N. The achievable accuracy decreases as the system size increases.

When a given problem can be formulated as a Fredholm integral equation of the second kind it may happen that its condition number is preserved. And even if the condition number is not exactly preserved, it does not depend on the number of discretization points (see Theorem 4.1.2 of Ref. [3] and Theorem 4.7.11 of Ref. [25]). Iterative schemes then converge rapidly and there is no need for preconditioners. The computing time grows linearly with N, given that fast solvers are used. The accuracy can be controlled. It should be pointed out that the preservation of condition number does not automatically follow from a Fredholm equation formulation. Neither does it exist a general recipe for the derivation of integral equations with this property. But for an increasing number of problems in science and engineering, usually formulated as partial differential equations, it has been possible to find reformulations where the condition number is almost preserved.

Despite the advantages mentioned above, integral equation methods are not standard for solving field problems. There are several reasons for this. One is historical. It was not until about 15 years ago that fast iterative solvers for integral equations in 2D first appeared [5,20,45]. Efficient codes for 3D have been finalized in the last few years [13,21]. Another reason, particularly relevant for solid mechanics, has to do with the difficulties of finding good integral equations for problems of interest. Sometimes, for simple geometries and linear problems, there are several equations to choose between, many of which were developed a long time ago and without numerical applications in mind. Comparative studies of the performance of these equations seem to be lacking. Often, for more complicated geometries, for problems with continuously varying material coefficients, and for non-linear problems, there are no equations available at all. Researchers working with efficient algorithms for large scale-problems do not always address these questions. They often concentrate on the speed of their solvers and then apply their algorithms to idealized problems. See, for example, Refs. [16,18,19,26, 54].

In other areas of scientific computing, such as scattering problems, integral equation methods have had much more of a revival [22]. Here their advantages over other methods are more easily exploited. Integral equations for problems of interest are available.

Other advantages with integral equations, often mentioned, but in our opinion less important than the stability properties, have to do with economy of points and ease of mesh-generation. For free-space problems and for problems involving piecewise homogeneous materials, integral equation methods often only require the discretization of the boundary.

It is clear, from above, that the present authors prefer to work with Fredholm integral equations of the second kind for stability reasons. It should be emphasized that most contemporary researchers working with integral equation solvers in solid mechanics use other approaches. The reason is that integral equations of the second kind are often very hard to find for a given problem. If one extends one's search to Fredholm equations of the first kind it is easier to find candidates. In fact, many popular integral equation used for fracture mechanics problems [8,37,44] (hypersingular extensions of the Somigliana identity [34], also known as standard BEM) are equations of this type. Schemes based on first kind equations retain some of the advantages of second kind integral equation schemes, such as ease of mesh-generation. Others are lost. Hypersingularity, which appears when double layer potentials are differentiated in the dual boundary boundary element method [8,44], is in some sense similar to differentiation—compare the Cauchy integral formula for an analytic function $f(z)$ and z inside Γ

$$\frac{1}{2\pi i} \int_\Gamma \frac{f(\tau)\mathrm{d}\tau}{(\tau - z)^2} = f'(z).$$

The properties of the hypersingular integral equation resemble those of a differential equation. In general, the condition number of system matrices that results from first kind equations have condition numbers that grow like $\mathrm{O}(N)$ [6]. For simple problems, solved with direct solvers, this is not a worry. For difficult problems and when iterative solvers are needed, the high condition number might negatively affect the accuracy of the solution and it may lead to unreasonably long execution times.

Because of the unbounded condition numbers for matrices of discretized first kind equations, preconditioning might be needed. There are several approaches for integral equations. Multigrid type preconditioners [43] and additive Schwarz preconditioners [1] are two examples which have

successfully been used for finite element equations. A preconditioning technique that is not an analogue to an existing finite element preconditioner, but takes advantage of the inherent properties of the integral equation, can be found in Steinbach and Wendland [49]. Here a discrete boundary integral operator of order $-m$ is used to precondition a boundary integral operator of order m. The idea is to mimic analytic left regularization. Time will tell if schemes using preconditioned integral equations will be competitive to differential equation based schemes such as the finite element method. The experience with solving truly large-scale problems is still very minimal.

In summary, numerical methods based on integral equations of Fredholm's second kind can be extremely stable. Their advantages over methods based on partial differential equations are most pronounced for large and ill-conditioned problems. Therefore, we argue, unless even more efficient numerical methods emerge [15], integral equation methods will become increasingly appreciated as computers get more powerful and more difficult problems are attacked. New integral equations need to be developed for solid mechanics problems.

3. Multiply connected finite 2D domains

In this section we review an integral equation for stress problems on multiply connected finite 2D domains. There are different opinions about what integral equation to use for this problem. Classic authors [40,41,48] recommend an approach starting with the Airy stress function and the Sherman–Lauricella representation. Another option is to use 'standard BEM', derived from the Kelvin fundamental solution to the elastostatic partial differential equation. Both these approaches, involving logarithmic kernels, have been reported to generate system matrices whose condition numbers grow with the system size [19,35]. Preconditioners are needed. We instead use a recently derived [29] non-singular Fredholm second kind equation which is free from this problem. Furthermore, the unknown quantity $\Omega(z)$ in the integral equation is the limit of an analytic function. This enables the use of high-order quadrature in the solution process also for polygonal domains.

3.1. The integral equation

A finite, linearly elastic, specimen occupies a domain D in the complex plane. The outer boundary of the specimen is denoted Γ_0 and is given positive (counter-clockwise) orientation. The domain D is multiply connected. Inside D there are a number N_h of holes. The holes have boundaries $\Gamma_j, j = 1, 2, ..., N_h$, and are given positive orientations. The union of all boundaries is Γ. The area of the region enclosed by contour Γ_j is A_j. Traction $t^{pr} = t_x^{pr} + it_y^{pr}$, that is, stress on the boundary, is prescribed at Γ_0. The holes are free of stress. The exterior of the domain is denoted D'.

Our integral equation [29] reads

$$(I - M_3 - 2iP_0)\Omega(z) = \frac{1}{2}\left(I - \frac{\bar{n}}{n}\overline{M}_1\frac{n}{\bar{n}}\right)\bar{n}t^{pr},$$

$z \in \Gamma_0,$ (1)

$$\left(I + M_3 + \frac{1}{2}P_ji(I + M_1) + i\bar{z}Q_j\right)\Omega(z) = \frac{1}{2}\frac{\bar{n}}{n}\overline{M}_1\frac{n}{\bar{n}}\bar{n}t^{pr},$$

$z \in \Gamma_j, \ j = 1, 2, ..., N_h.$ (2)

Here I is the identity and z is a point in the complex plane. The operator M_3 can be regarded as compact, see Section 4.2. Its action on a function $f(z)$ is given by

$$M_3f(z) = \frac{1}{2\pi i}\left[\int_\Gamma \frac{f(\tau)d\tau}{(\tau - z)} + \frac{\bar{n}}{n}\int_\Gamma \frac{f(\tau)d\tau}{(\bar{\tau} - \bar{z})} + \int_\Gamma \frac{\overline{f(\tau)}d\bar{\tau}}{(\bar{\tau} - \bar{z})}\right.$$
$$\left. + \frac{\bar{n}}{n}\int_\Gamma \frac{(\tau - z)\overline{f(\tau)}d\bar{\tau}}{(\bar{\tau} - \bar{z})^2}\right], \qquad z \in \Gamma,$$ (3)

where $n = n_x + in_y$ is the outward unit normal vector at z on Γ. The operator P_j is a mapping from Γ_j to $\mathbb{R}$, defined by

$$P_jf = -\frac{1}{2A_j}\mathscr{R}e\left\{\int_{\Gamma_j} f(\tau)\bar{\tau}\,d\tau\right\}.$$ (4)

The Cauchy singular operator M_1, acting on a function $f(z)$, is given by

$$M_1f(z) = \frac{1}{\pi i}\int_\Gamma \frac{f(\tau)d\tau}{(\tau - z)}, \qquad z \in \Gamma.$$ (5)

The conjugate of M_1 is denoted $\overline{M}_1$. The operator Q_j is a mapping from Γ_j to $\mathbb{C}$, defined by

$$Q_jf = -\frac{1}{2A_j}\int_{\Gamma_j} f(\tau)d\tau.$$ (6)

3.2. Extraction of useful quantities from the solution

Once Eqs. (1) and (2) are solved for $\Omega(z)$ several quantities of physical interest can be extracted. For example, the components of the stress tensor are

$$\sigma_{xx} + \sigma_{yy} = 4\mathscr{R}e\{\Phi(z)\},$$ (7)

$$\sigma_{yy} - \sigma_{xx} - 2i\sigma_{xy} = 2(z\overline{\Phi'(z)} + \overline{\Psi(z)}),$$ (8)

where $\Phi(z)$ and $\Psi(z)$ are analytic functions in $D \cup D'$ given by

$$\Phi(z) = \frac{1}{2\pi i}\int_\Gamma \frac{\Omega(\tau)d\tau}{(\tau - z)}, \qquad z \in D \cup D',$$ (9)

and

$$\Psi(z) = -\frac{1}{2\pi i}\int_\Gamma \frac{\overline{\Omega(\tau)}\mathrm{d}\bar\tau}{(\tau - z)} - \frac{1}{2\pi i}\int_\Gamma \frac{\bar\tau\overline{\Omega(\tau)}\mathrm{d}\tau}{(\tau - z)^2}$$
$$\qquad -\frac{1}{2\pi i}\int_{\Gamma_0}\frac{\bar n\overline{t^{\mathrm{pr}}}\,\mathrm{d}\tau}{(\tau - z)}, \qquad z \in D \cup D'. \tag{10}$$

A quantity of particular interest for perforated structures is the normalized tangential stress $\hat\sigma_t(z)$. One can define $\hat\sigma_t(z)$, for a stress-free hole, as the ratio of the trace of the stress tensor on the hole to the trace of the applied stress tensor. This quantity is easy to compute since $\Omega(z)$ is minus the outside limit of $\Phi(z)$ on the holes [29]. With external uniform unit traction applied in the y-direction we get

$$\hat\sigma_t(z) = -4\mathscr{R}e\{\Omega(z)\}, \qquad z \in \Gamma - \Gamma_0. \tag{11}$$

A stress concentration factor K_t can be defined as the maximum absolute value of the normalized tangential stress

$$K_t = \max_{z \in \Gamma - \Gamma_0}|\hat\sigma_t(z)|. \tag{12}$$

Should Γ_0 contain notches, it could be of interest to compute so-called notch stress intensity factors. The most common of these is the dimensionless mode I notch stress intensity factor Q_I, defined in a Cartesian coordinate system, see Fig. 1, as

$$Q_I \equiv \lim_{x\to 0^+}\sqrt{2\pi}\frac{\sigma_{yy}}{\sigma_{\mathrm{ref}}}\left(\frac{x}{L}\right)^{1-\lambda_1}, \tag{13}$$

where L is a characteristic length scale, σ_{ref} is a reference stress, and the limit is taken along the x-axis. For a single edge notched specimen under uniform uniaxial load, such as the one depicted in Fig. 2, a common choice is to take L as the width w of the specimen and σ_{ref} as the magnitude of the prescribed traction. The exponent λ_1 in Eq. (13) is the first exponent in a local Williams series expansion [28,51] of the field $\Phi(z)$ in the notch (the re-entrant corner E of Fig. 2)

$$\Phi(z) = \sum_n f_n z_*^{\lambda_n-1} + \bar f_n \bar z_*^{\bar\lambda_n-1} + g_n z_*^{\mu_n-1} - \bar g_n \bar z_*^{\bar\mu_n-1}, \tag{14}$$

where f_n and g_n are real or complex coefficients [28,51] and z_* is the position in a local coordinate system with origin at the notch and oriented as in Fig. 1. The symmetric and
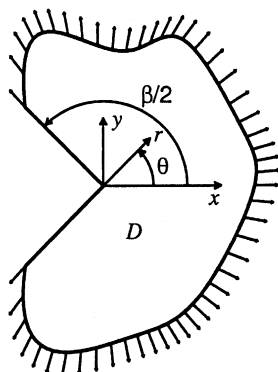

Fig. 1. A domain D with a traction-free notch. The included vertex angle is β. The notch tip is at the origin. Traction is applied outside the notch.
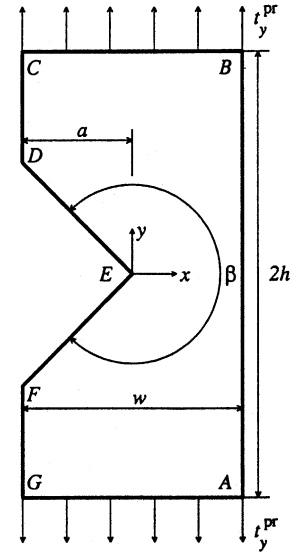

Fig. 2. A generic single edge notched specimen of length $2h$ and width w. The notch depth is a. The included vertex angle is β. Uniform uniaxial traction t_y^{pr} is prescribed.

anti-symmetric exponents λ_n and μ_n are given by the infinite number of solutions to the trigonometric equations

$$\lambda\sin(\beta) + \sin(\lambda\beta) = 0, \tag{15}$$

$$\mu\sin(\beta) - \sin(\mu\beta) = 0. \tag{16}$$

In Section 4, we discretize Eqs. (1) and (2) according to a Nyström scheme with composite quadrature. The corners will be symmetrically included in corner panels, each with two legs of length l_{cp}. If the expansion of Eq. (14) is done in a local coordinate system where the corner legs have length unity, the notch stress intensity factor Q_I of Eq. (13) can be computed as

$$Q_I = \frac{f_1}{\sigma_{\mathrm{ref}}}2\sqrt{2\pi}(\lambda_1+1)\left(\frac{l_{\mathrm{cp}}}{w}\right)^{1-\lambda_1}\left[1 - \frac{\cos((\lambda_1-1)\beta/2)}{\cos((\lambda_1+1)\beta/2)}\right]. \tag{17}$$

There also exists a possibility to evaluate the notch stress intensity factor of Eq. (13) as a path-independent integral which encloses the notch [2,50]. An advantage with this post-processor is that it is less sensitive to the quality of the solution $\Omega(z)$ close to the notch than the formula (17). In fact, it can give reasonable estimates even if the solution at a few discretization points close to the notch-tip is completely wrong [50]. A drawback with the path-independent integral is that it is difficult to implement in an automated fashion. Since our scheme gives a rather accurate solution $\Omega(z)$ also close to the notch, see Section 4.4, we prefer the much simpler formula (17).

4. Implementation

In this section we present details on our numerical algorithm for solving Eqs. (1) and (2). We try to be brief,

yet detailed enough so that the general ideas will come through clearly.

4.1. The mesh

The integral equations (1) and (2) are to be discretized according to a Nyström scheme with composite quadrature. First, the boundary Γ is divided into quadrature panels of approximately equal lengths. The panels are placed in such a way that whenever a panel contains a corner, the corner must be at the center of the panel. Corners containing panels are called corner panels. The two halves of the corner panels are called legs. The remaining panels are called regular panels. The solution $\Omega(z)$ will be singular on corner panels and smooth on regular panels. We intend to use polynomial quadrature on regular panels and a quadrature which takes the singular nature of the solution into account on corner panels. There is an extra need for resolution on regular panels neighboring to corner panels. Therefore these panels are subdivided twice. See Fig. 3 for an example where panels are placed on the boundary of the specimen in Fig. 2.

We intend to use Gauss–Legendre quadrature on the regular panels. The discretization points z_j are placed accordingly. On the corner panels we intend to use a quadrature based on the expansion (14) and extra resolution of the kernels of M_1 and M_3, see Section 4.3. The number of discretization points for $\Omega(z)$ on a corner panel is related to how many terms that are used in expansion (14). The mapping from pointwise complex values of $\Omega(z)$ at the discretization points on a corner panel to the coefficients f_n and g_n preceding the basis functions $z_{*j}^{\lambda_n-1}, z_{*j}^{\bar{\lambda}_n-1}, z_{*j}^{\mu_n-1}, z_{*j}^{\bar{\mu}_n-1}$ of Eq. (14) is given by inverting the relation
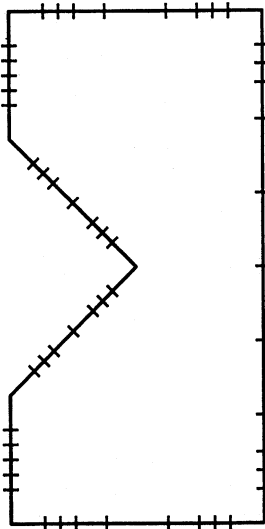

Fig. 3. The boundary of the single edge notched specimen of Fig. 2 is divided into quadrature panels. First, all panels are given approximately the same lengths. Then, panels neighboring to corner panels are subdivided twice.

$$\Omega(z_j) = \sum_{n=1}^{N_p/2+m_1} \mathscr{R}e\{f_n\}\left(z_{*j}^{\lambda_n-1}+z_{*j}^{\bar{\lambda}_n-1}\right) + \sum_{n=1+m_2}^{N_p/2+m_2-m_1} i\mathscr{I}m\{f_n\}$$

$$\times\left(z_{*j}^{\lambda_n-1}-z_{*j}^{\bar{\lambda}_n-1}\right) + \sum_{n=1+m_4}^{N_p/2+m_4-m_3} \mathscr{R}e\{g_n\}\left(z_{*j}^{\mu_n-1}-z_{*j}^{\bar{\mu}_n-1}\right)$$

$$+ \sum_{n=1}^{N_p/2+m_3} i\mathscr{I}m\{g_n\}\left(z_{*j}^{\mu_n-1}+z_{*j}^{\bar{\mu}_n-1}\right),$$

$$j=1,2,...,N_p, \tag{18}$$

where N_p is the number of discretization points on the corner panel and the exponents λ_n and μ_n are, for traction-free corners, the solutions to Eqs. (15) and (16). For corners that are not traction-free, but loaded with a uniform traction, the series λ_n need to be completed with the exponent $\lambda=1$. The integers m_2 and m_4 in Eq. (18) denote the number of exponents λ_n and μ_n which are real valued. The numbers m_1 and m_3 are chosen so that $2m_1 \approx m_2$ and $2m_3 \approx m_4$, that is, so that the series for real and imaginary parts of f_n and g_n are truncated after approximately the same number of terms. The number N_p will be chosen as an even number large enough so that the first omitted term in the series for λ_n and μ_n is approximately equal for all corners. When the exponents λ_n and μ_n lie widely spaced, we complete these series with integer exponents 1, 2, 3, etc. so that we get at least $N_p=8$ points on all corner panels.

4.2. Avoiding singular operators

Each of the four integrals appearing in the expression (3) for M_3 are singular and should be interpreted in Cauchy principal value sense. Fortunately, no Cauchy principal values need to be computed in the evaluation of $M_3\Omega(z)$. This is so since the sum of the kernels in the first and in the second integral of M_3 have the same (finite) left- and right-limits as $\tau \to z$, at least for twice continuously differentiable curves, see also Remark 4.1. Similarly, the sum of the kernels in the third and in the fourth integral of M_3 have the same (finite) left- and right-limits as $\tau \to z$. No discretization points are placed in corners. Therefore, the kernel of the operator M_3 can be regarded as continuous and the operator is compact. The limiting values of the kernel, when $\tau = z$, are calculated analytically. The only problem with evaluating the kernel of M_3 occurs when τ is close to z. Then some cancellation will take place.

The kernel of M_1 in Eq. (5) is clearly singular if z and τ are on the same quadrature panel. If z and τ are on different panels the kernel is smooth and the operator is compact. The operator M_1 appears in three places in Eqs. (1) and (2)—twice on the right-hand side and once on the left-hand side. In this paper, for simplicity and reasons of accuracy, we avoid numerical evaluation of M_1 acting on $\Omega(z)$ when M_1 is singular. (In earlier work we implemented M_1

numerically [28,29].) This is achieved as follows: the action of M_1 on the piecewise constant traction t^{pr} on the right-hand side of the equations is evaluated analytically. The action of M_1 on $\Omega(z)$ on the left-hand side of Eq. (2) followed by application of P_j, when τ and z are on the same hole, is evaluated by changing the order of integration

$$-\mathscr{R}e\left\{\int\!\!\int_{\Gamma_j}\frac{\bar{\tau}\,\mathrm{d}\tau}{\pi}\int_{\Gamma_j}\frac{\Omega(z)\mathrm{d}z}{z-\tau}\right\}$$

$$=\mathscr{R}e\left\{\int\!\!\int_{\Gamma_j}\frac{\Omega(z)\mathrm{d}z}{\pi}\int_{\Gamma_j}\frac{\bar{\tau}\,\mathrm{d}\tau}{\tau-z}\right\}. \tag{19}$$

The last integral on the right-hand side of Eq. (19) will be evaluated analytically for circular holes. For holes of arbitrary shapes it needs to be evaluated numerically. It is an easier task to evaluate the action of singular integral operators accurately on functions which are known everywhere, than on functions which are known only at a set of nodes.

Remark 4.1. Eq. (1) was originally derived under the assumption of a smooth Γ_0 and for the more regular potential $\phi(z)$, see paragraph 98 of Muskhelishvili [41]. The operator M_3 is compact under the smoothness assumption. In corners the boundary Γ_0 is merely continuous. As a consequence, the potential $\Phi(z)$ of Eq. (9) has a limiting value on Γ_0 from the inside (the unknown quantity $\Omega(z)$ in Eq. (1)) which in corners may not even be finite. Fortunately, interpreted in a suitably generalized sense, Eq. (1), also holds for domains with corners, see paragraph 99 and 100 of Muskhelishvili [41]. More precisely, the corner points should be excluded from the domain of validity and integrals for which bounded primitive functions exist should be interpreted as generalized Riemann integrals.

4.3. Quadrature

This section describes the quadrature used in our discretization scheme. We discuss the quadrature in the

context of the integral operator M_3 acting on $\Omega(z)$. The operator M_1 poses no extra difficulties, see Section 4.2.

A Nyström scheme means that $\Omega(z)$ is represented by pointwise values on Γ, and that Eqs. (1) and (2) should be satisfied at those same discretization points. We adopt the term *source points* for the discretization points where $\Omega(z)$ has support, and the term *target points* for the discretization points where Eqs. (1) and (2) should be satisfied. Obviously, source points and target points are the same points in a Nyström scheme. Still, it helps to distinguish between the two uses of the points when discussing quadrature techniques. We use the variable τ when we emphasize the source point aspect, and the variable z when we emphasize the target point aspect of a point. The kernel of M_3 will be referred to as $M(z,\tau)$.

On regular panels, the density $\Omega(\tau)$ and the kernel $M(z,\tau)$ are smooth functions of τ. They can be well approximated with polynomials. We use 10-point Gauss–Legendre quadrature for the discretization of $M_3\Omega(z)$, and we place the discretization points accordingly, see the left image of Fig. 4.

On the corner panels, the density $\Omega(\tau)$ is singular. The nature of $M(z,\tau)$ depends strongly on whether the target point z is on, close to, or far removed from the corner panel. The basic strategy is to switch representation for $\Omega(\tau)$ from N_p complex pointwise values to $2N_p$ real and imaginary parts of coefficients in expansion (18). Then we evaluate the integrals $M_3\phi_n(z)$, where $\phi_n(\tau)$ are the $2N_p$ basis functions in Eq. (18). Finally, we multiply the coefficients with the corresponding integrals.

When a target point z is far away from a corner panel and the source points τ are on a leg of a corner panel, $M(z,\tau)$ is a smooth function of τ. For the evaluation of $M_3\phi_n(z)$ we use simple 16-point quadrature, based on the values of $M(z,\tau)$ and $\phi_n(\tau)$ at 16 temporary points on each corner leg. As for the placement of these temporary points, the Gauss–Legendre points seems as a reasonable choice.

When target points z are close to, or on, a corner panel and the source points τ are on a corner panel, $M(z,\tau)$ is not a smooth function of τ. We use adaptive quadrature and evaluate a number of integrals of which we give one particular example. We introduce a parameterization t for
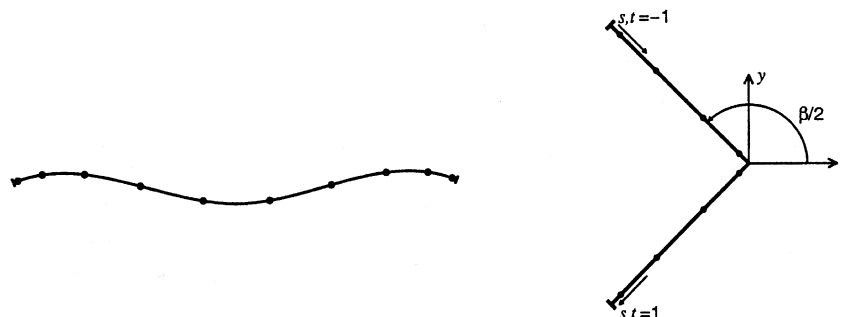

Fig. 4. Left: a regular quadrature panel. Right: a corner quadrature panel with two legs. The dots symbolize points where the solution $\Omega(z)$ has support.

the local coordinate τ_*, and a parameterization s for the local coordinate z_*, see the right image of Fig. 4. Now τ_* and z_* (on the corner panel) range from $e^{i\beta/2}$ to $e^{-i\beta/2}$ as t and s range from -1 to 1. The action of M_3 on the basis function $\tau_*^{\lambda_n-1}$ for z on the first leg can be computed as

$$M_3\tau_*^{\lambda_n-1}(z) =$$
$$e^{-i\beta(\lambda_n-1)/2}\left[\frac{(1-\bar{n}_z^2 n_\tau^2)}{2\pi i}\int_0^1\frac{t^{\lambda_n}\,dt}{(t^2+s^2-2\mathscr{R}e\{\bar{n}_z n_\tau\}ts)}\right]$$
$$+ e^{i\beta(\bar{\lambda}_n-1)/2}\left[\frac{(1-\bar{n}_z^2 n_\tau^2)}{2\pi i}\int_0^1\frac{t^{\bar{\lambda}_n}\,dt}{(t-\bar{n}_z n_\tau s)^2}\right],$$
$$s\leq 0, \tag{20}$$

where $n_z = ie^{i\beta/2}$, and $n_\tau = -ie^{-i\beta/2}$. Similarly, for a target point z on the second leg we get

$$M_3\tau_*^{\lambda_n-1}(z) =$$
$$-e^{i\beta(\lambda_n-1)/2}\left[\frac{(1-\bar{n}_z^2 n_\tau^2)}{2\pi i}\int_0^1\frac{t^{\lambda_n}\,dt}{(t^2+s^2+2\mathscr{R}e\{\bar{n}_z n_\tau\}ts)}\right]$$
$$\tag{21}$$
$$-e^{-i\beta(\bar{\lambda}_n-1)/2}\left[\frac{(1-\bar{n}_z^2 n_\tau^2)}{2\pi i}\int_0^1\frac{t^{\bar{\lambda}_n}\,dt}{(t+\bar{n}_z n_\tau s)^2}\right], \qquad 0\leq s,$$

where now $n_z = -ie^{-i\beta/2}$ and $n_\tau = ie^{i\beta/2}$. This example was also given in Ref. [28], along with a more thorough derivation. Unfortunately, there was a misprint in Ref. [28], which is corrected here.

4.4. Properties of the quadrature

In Section 4.3 we reviewed the quadrature techniques used. Here we comment on some of the properties of the quadrature and discuss theoretical aspects.

The problem of solving for stress on a multiply connected polygonal domain is inherently well conditioned. With this we mean that a small change in the applied load or in some overall geometric descriptor, such as an included vertex angle, will lead to small changes in most quantities computed from the solution, such as notch stress intensity factors [23] and the pointwise stress-field inside the domain. The problem is, however, ill-conditioned in the sense that a small relative perturbation of a local geometric descriptor, such as the position of a discretization point close to a corner, can lead to a large change in the solution close to that corner. Therefore, even with Fredholm integral equations of the second kind, it is difficult to construct entirely stable algorithms. Great care must be taken to compute geometric quantities as accurately as possible. The dominant source of error, in our algorithm, comes from the corner quadrature. Errors arise, for example, when we invert relation (18). The ill-conditioning of this operation increases with the number of discretization points and terms that are included, and with the spacing of the exponents λ_n and μ_n of Eqs. (15) and (16). There is, thus, a trade-off between

convergence and stability: the more exponents that are included—the more rapid convergence but the lower achievable accuracy in the solution close to corners. In this paper, we have decided to include exponents λ_n and μ_n which have a real part less than approximately seven. For the problems studied in this paper this means that N_p will range from 8 to 12.

Let us now consider the placement of the N_p discretization points on a corner panel. For Gaussian quadrature, a correct placement of the nodes ensures the Gaussian property that $2N$ basis functions are integrated exactly. For the quadrature described in Section 4.3, any distinct placement of N_p discretization points on a corner panel will give $2N_p$ coefficients and enable exact integration of $2N_p$ basis functions for $\Omega(\tau)$. Unfortunately, we are not interested in merely integrating $\Omega(\tau)$, but the composition $M_3\Omega(z)$. For this we need to evaluate $\phi_n(\tau)$ at other points than the discretization points, as described in Section 4.3. Anyhow, the placement of the discretization points on a corner panel turns into N_p free parameters which could be chosen as to fine tune the algorithm. One option would be to choose this freedom to minimize the error, in some appropriate norm, of the approximation of $\Omega(\tau)$ with a linear combination of $2N_p$ basis functions $\phi_n(\tau)$. Another option would be to choose the placement of the N_p points as to minimize the condition number of the mapping (18). In this paper, lacking a good theory, we have chosen to place the N_p discretization points symmetrically on the corner panels. On each leg the nodes are placed according to a Gauss–Lobatto quadrature rule for integrals of the type

$$\int_0^1\frac{f(x)}{\sqrt{x}}\,dx,$$

where $f(x)$ is a smooth function, and the node $x = 1$ is fixed.

The determination of which target points z should be considered as being 'far away' from the corner panels must also be made. In the setups studied in this paper the kernel $M(z,\tau)$ is sufficiently smooth as to be accurately evaluated with 16 points on each corner leg when the point z is located a distance further than $4l_{cp}$ away from the corner panel.

4.5. Precomputation and variable precision

Some quantities needed in the algorithm are precomputed in extended (Fortran `REAL*16` and `COMPLEX*32`) precision. These quantities are then rounded to double (`REAL*8` and `COMPLEX*16`) precision and included in the main program. Precomputation of a limited number of quantities is standard in integral equation based solvers [53]. This practice can both save time and increase the stability of the code. Naturally, the more quantities that are precomputed, the more stably the code will run. If we were to precompute all quantities related to corner quadrature in extended precision, our problems with ill-conditioning would go away. This would take time.

Precomputation in extended precision is equivalent to a special case of variable precision arithmetic where all unknowns and most geometric quantities are declared in double precision, and a few geometric quantities and intermediary variables are declared in extended precision. While, perhaps, unorthodox, variable precision coding could be a simplification over regular precomputation. It eliminates the need for reading and writing on files. The composition of matrices, declared in higher precision, does not need to be carried out explicitly. We believe that variable precision arithmetic could be efficient for particularly difficult problems. Section 5.1 gives a few examples where extended precision is used for the intermediary variables and geometric quantities appearing on the right-hand side of Eq. (18). In most numerical examples we use double precision throughout the main program and only precompute, in extended precision, a few quantities such as Gaussian weights and nodes, and inverses of small matrices.

4.6. Iterative solution and fast multipoles

We solve the discretized systems (1) and (2) iteratively using the GMRES solver [46]. Compensated summation [31,36] is used whenever deemed beneficial, for example, when computing inner products. The matrix–vector multiplications will be done in a direct way for smaller systems, and with the use of the fast multipole method [5,20, 45] for larger systems. The only part of the action of M_3 on $\Omega(\tau)$ which cannot be computed with the fast multipole method is the evaluation of $M_3\phi_n(z)$, when z is close to, or on, a corner panel. The computational work for these integrals scales linearly with the number of corners.

The fast multipole method has previously been used in 2D elastostatic contexts, involving operators similar to M_1 and M_3, see Refs. [17–19,26]. The basic strategy is to compute the fields $\Phi(z)$ of Eq. (9) and $\Psi(z)$ of Eq. (10) and then to combine the results as to get $M_1\Omega(z)$ and $M_3\Omega(z)$. The computation of $M_3\Omega(z)$ requires about 70% more work than the mere computation of $M_1\Omega(z)$. In the present code, for simplicity, we use a non-adaptive implementation based on the original scheme of Ref. [20] but which includes some ideas of Ref. [32]. For example, the number of levels of refinement in the hierarchical mesh is not $L = \log_4 N$, where N is the number of discretization points, but is determined by the condition that no box on the finest mesh level should contain more than $p = -\log_2 tol$ points, where tol is the requested relative precision. For a uniform distribution of points, this reduces the complexity in precision from p^2 to p, see Fig. 5 and compare Step 1 of Ref. [32]. Also, the costly conversions of multipole expansions into local Taylor
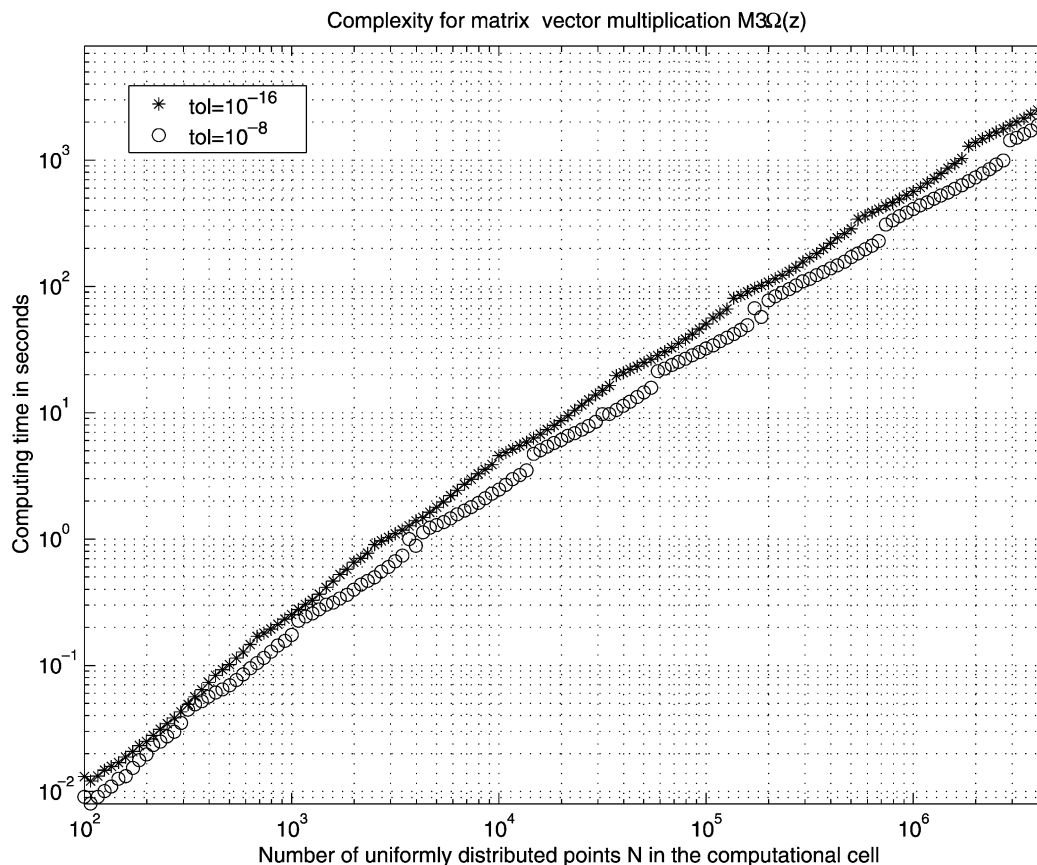

Fig. 5. Computing time on a SunBlade 100 workstation for one matrix–vector multiplication $M_3\Omega(z)$ as a function of the number of uniformly (randomly) distributed points N in the computational cell. The field $M_1\Omega(z)$ is computed as a partial result in this process. Stars refer to a requested relative precision of $tol = 10^{-16}$ and $p = 53$ terms in multipole and local Taylor expansions. Open circles refer to $tol = 10^{-8}$ and $p = 27$.

expansions for boxes in the interaction list of a box *b* are replaced by direct evaluation of multipole fields at points in *b* if the number of such points is smaller than *p*/2, compare Step 5 of Ref. [32]. We refrain from giving more details since our code is not entirely optimal with respect to the latest development in the field. More efficient and consequent implementations for some kernels in 2D do exist [32,15]. We speculate that speedups with a factor of 5–10 are possible for the problem sizes treated in this paper.

The number of GMRES iterations needed to solve the discretized systems (1) and (2) to a given relative precision in the residual, chiefly depends on the geometry and on the load. It does not depend on the total number of discretization points. This is a consequence of Eqs. (1) and (2) being a Fredholm integral equation of the second kind. The number of GMRES iterations needed does, however, depend mildly on the number of exponents λ_n and μ_n included in expansion (18). More exponents increase the ill-conditioning of the algorithm, see Section 4.4.

4.7. Order of accuracy

In a Nyström scheme, the order of convergence of the unknown quantity is determined by the (worst) quadrature used in the discretization of the integral equation. The order of convergence of post-processed quantities may, of course, also depend on the order of accuracy of the post-processor. In our scheme, the order of convergence for $\Omega(z)$ will, asymptotically, be determined by the magnitude of the smallest exponent λ_n or μ_n omitted in expansion (18). The notch stress intensity factor Q_I of Eq. (17) is directly computed from $\Omega(z)$ in the corners and will have the same order of convergence. The stress field inside *D* is computed as integrals over $\Omega(z)$. The asymptotic order of convergence for the stress field at a given point in *D* will be of the order one higher than for Q_I since the quadrature has higher order on regular panels than on corner panels, and since the relative number of corner panels decreases as the mesh is refined. The same holds true for the stress concentration factor K_t of Eq. (12). We compute K_t in a post-processor using Newton's method, safeguarded with Golden Section Search. This procedure is, in itself, 9th order accurate.

5. Numerical examples

This section presents numerical examples. We start with some simple standard setups in order to verify the properties of our algorithm, stated in Section 4, and to compare with previous results. We end with a rather complex, large-scale computation.

5.1. Some SENT specimens

The notch stress intensity factor Q_I of Eq. (13) of a single edge notched specimen under uniaxial tension (SENT), such

as the one in Fig. 2, has been computed by numerous researchers during the last 30 years. A common setup has a height to width ratio of $h/w = 1$, a relative notch depth of $a/w = 0.5$ and an included vertex angle of $\beta = 3\pi/2$. A typical agreement between the results of different authors is three digits, see Ref. [28], Table 1. The most accurate computation, $Q_I = 4.2959$, was done by Strandberg [50].

We performed four sets of computations, presented in Fig. 6. First we used $N_p = 8$ points on all corner panels. The smallest exponent λ_n or μ_n not fully included in the expansion of Eq. (18) was λ_5 in the corner *E* of Fig. 2. The real part of this exponent is approximately 5.65, which corresponds well with the slope of the line formed by the open circles in Fig. 6. Next, we increased the number of points on the corner panel at corner *E* to $N_p = 10$, while all other corner panels still have $N_p = 8$. The smallest exponent not fully included in the expansion of Eq. (18) is now λ_6 in the corner *E*. The real part of this exponent is approximately 6.98. The stars in Fig. 6 show that the convergence now is so rapid that we do not reach the asymptotic regime, in IEEE DP arithmetic, until the instabilities in the algorithm start to pollute the result. Therefore, we also performed variable precision arithmetic computations, see Section 4.5, and full quadruple precision computation, shown as triangles and diamonds in Fig. 6, respectively. The asymptotic regime is reached at about 3000 discretization points and at a relative precision in Q_I of about 10^{-16}. The number of GMRES iterations needed in all these computations was around 40.

We also include results for some SENT specimens with a larger included vertex angle β. These geometries are considered more difficult to treat [50]. The largest angle for which we have found computations in the literature is $\beta = 11\pi/6$. We solve for a few setups with $\beta = 11\pi/6$ using a number of $N_p = 12$ points on the corner panel containing the notch, and compare with previous results. Gross and Mendelson [23] report a value of $Q_I = 3.569$ for a setup with $a/w = 0.5$ and $h/w = 1.2$. At about 1600 discretization points our computations converge to a value of $Q_I = 3.5696698583(19)$, where the two digits within parenthesis are obtained using variable precision arithmetic. Strandberg [50] report a value of $Q_I = 3.5696$ for a setup with $a/w = 0.5$ and $h/w = 2$. At about 2300 discretization points our computations converge to a value of $Q_I = 3.5696265724(35)$. Chen [11] report a value of $Q_I = 3.568$ for a setup with $a/w = 0.5$ and $h/w = 3$. At about 3000 discretization points our computations converge to a value of $Q_I = 3.5696260183(43)$. The number of GMRES iterations needed in all these computations was around 60.

5.2. A square with one hole

A very simple finite and multiply connected domain is the unit square with a circular hole of radius *R* centered at the origin. The problem of determining the stress concentration factor K_t of Eq. (12) for the hole, when a uniaxial stress is applied, has been treated by Isida and Sato [33] and
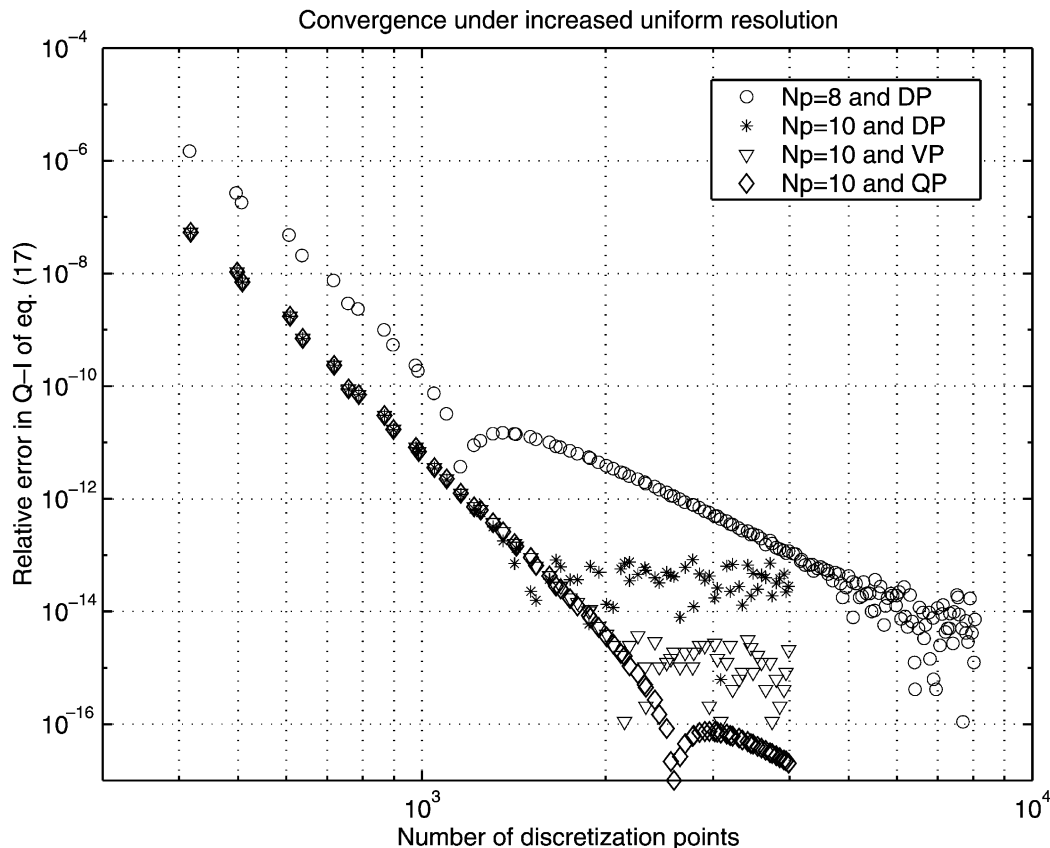
Fig. 6. Convergence of the notch stress intensity factor Q_I of Eq. (17) for the setup in Fig. 2. The geometry is chosen so that $h/w = 1$, $a/w = 0.5$ and $\beta = 3\pi/2$. The reference value is chosen as $Q_I = 4.2958869676986369$. DP refers to double precision arithmetic, VP to variable precision arithmetic, and QP to quadruple precision arithmetic. N_p is the number of discretization points on the corner panel containing the notch.
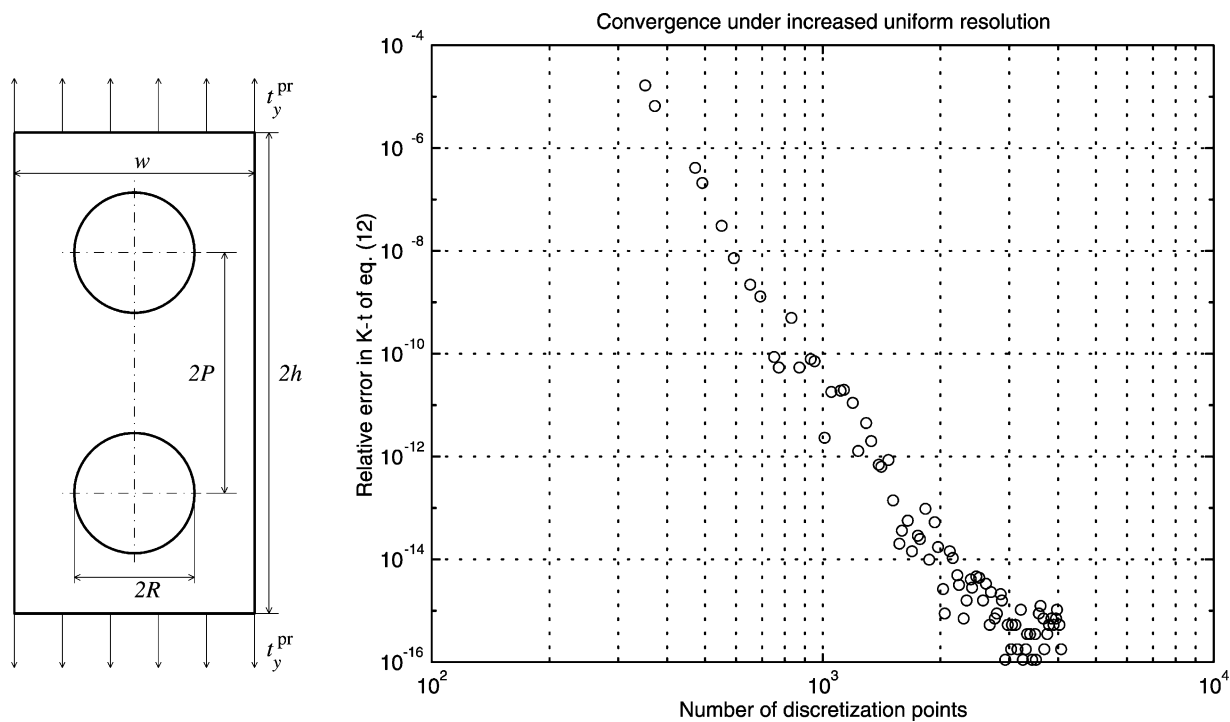

Fig. 7. Left: two symmetrically placed holes of radius R separated a distance $2P$ in a rectangular plate with height $2h$ and width w. Uniaxial stress t_y^{pr} is prescribed at two opposing sides. Right: convergence of the stress concentration factor K_t of Eq. (12). The geometry is chosen so that $h/w = 1$, $2P = w$ and $R = w/4$. The reference value is chosen as $K_t = 5.0438755094142715$. The computations are done in double precision arithmetic.
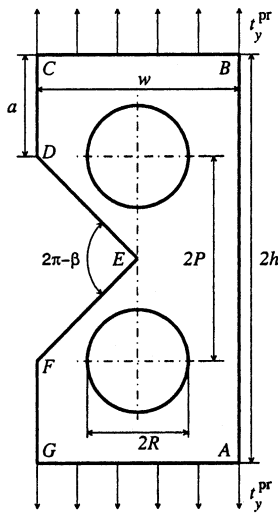
Fig. 8. Two symmetrically placed holes of radius R separated a distance $2P$ in a single edge notched specimen with height $2h$ and width w. The notch depth is a. The included vertex angle is β. Uniaxial stress t_y^{pr} is prescribed at two opposing sides.

by Nisitani and Chen [42]. For a hole with radius $R = 0.25$ they report the values $K_t = 6.3887$ and 6.38869, respectively. Our computations reproduce their values. With 2000 discretization points we get convergence to $K_t = 6.3886960194568$.

5.3. A rectangle with two holes

A rectangle with two holes is depicted in the left image of Fig. 7. We choose the same height to width ratio as for the first SENT specimen in Section 5.1, that is, $h/w = 1$. We take the separation distance to be $2P = w$ and the radius to be $R = w/4$. Double precision arithmetic is used throughout the code. A convergence study for the stress concentration factor K_t of Eq. (12) is presented in the right image of Fig. 7.

Note that the problem is well-conditioned and that we do not reach the asymptotic regime: the convergence rate is dominated by the 9th order error in the optimization process, see Section 4.7.

5.4. A SENT specimen with two holes

We now combine the geometries of Figs. 2 and 7, left image, to get the perforated SENT specimen of Fig. 8. As before, we take $h/w = 1$, $2P = w$, $R = w/4$, $a/w = 0.5$ and $\beta = 3\pi/2$. We perform one set of computations for Q_I and K_t where the matrix–vector multiplications are done in a direct way, see the left image of Fig. 9, and one set of computations where the matrix–vector multiplications are accelerated with the fast multipole method, see the right image of Fig. 9. Note that the direct computations give a couple of extra correct digits. The direct computations, involving the actual construction of the discretized matrix M_3, converge better than the multipole accelerated computations since we have taken advantage of that some of the entries in M_3 can be expressed on analytic form, and since we used compensated summation for the direct matrix–vector multiplications.

5.5. A SENT specimen with 1170 holes

Lastly, in a large-scale example, we increase the number of holes in the SENT specimen of Fig. 8 substantially. Again we take $h/w = 1$, $a/w = 0.5$ and $\beta = 3\pi/2$. We then construct a square grid on the specimen. The origin is at the notch-tip E. The grid points are separated a distance d from their nearest neighbors and placed at coordinates given by $x = 0.5d + id$ and $x = 0.5d + jd$ where i and j are integers. The separation distance d is given by $d = w/n$ where n is an even integer. Grid points on the boundary are removed. Now
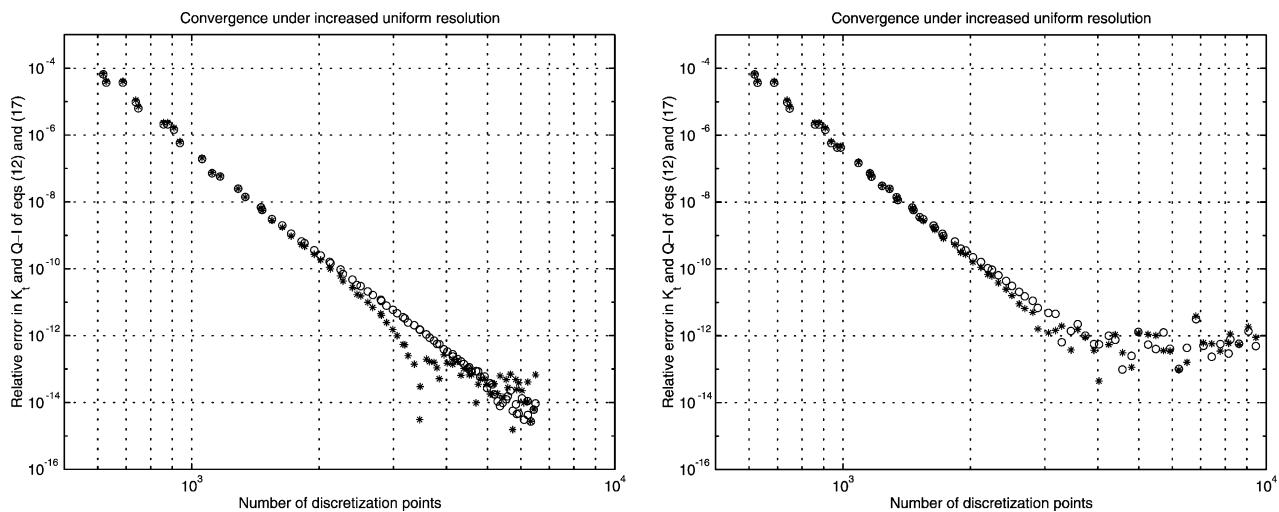

Fig. 9. Convergence of the notch stress intensity factor Q_I of Eq. (17), shown as stars, and the stress concentration factor K_t of Eq. (12), shown as open circles, for the setup in Fig. 8. Left: matrix–vector multiplications are computed in a direct manner. Right: matrix–vector multiplications are accelerated with the fast multipole method. Reference values are taken as $Q_I = 5.1691735817822$ and $K_t = 15.10033207977245$.
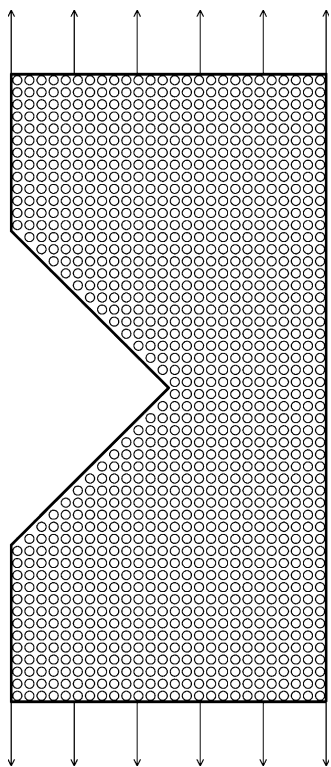
Fig. 10. A number of 1170 equisized circular holes in the SENT specimen of Fig. 2. The holes are placed on a square grid. The radii of the holes are $3/8$ of their separation distance. Uniaxial stress t_y^{pr} is prescribed at two opposing sides.

we place a circular hole with radius $R = 3d/8$ at each grid point. There will be $7n^2/4 - n/2$ such holes. We choose $n = 26$ and get 1170 holes, see Fig. 10. A convergence study of the quantities Q_I and K_t is given in the left image of Fig. 11. We refined the mesh until our workstation ran out of memory. The number of GMRES iterations needed for full convergence was about 110 and

independent of the number of discretization points. See the right image of Fig. 11 for an iteration history.

One could expect that the achievable accuracy of Q_I and K_t should be lower in the present large-scale example than in the small-scale example of Section 5.4. After all, a hundred times more discretization points and closer spacing between the points on the boundary would lead to larger errors. One can see, by comparing the left image of Fig. 11 with the right image of Fig. 9, that the convergence in this large-scale example is almost as stable as the convergence in the much smaller example. Our explanation for this is that both the notch and the hole, for which the maximum in Eq. (12) occurs, are located close to the origin where the relative accuracy of computed positions is higher than elsewhere in the computational cell.

6. Conclusions and discussion

This paper presents numerical results for stress computations on multiply connected finite polygonal domains. The algorithm is based on the second kind Fredholm integral Eqs. (1) and (2) which are discretized using a Nyström scheme and composite quadrature. The analytic properties of the unknown quantity enable the use of high order quadrature. Pointwise representation enables easy incorporation of the fast multipole method into an iterative solver. Computations of notch stress intensity factors and stress concentration factors are obtained from the solution via simple post-processors. Convergence to high accuracy was demonstrated for a SENT perforated with 1170 circular holes.

Eqs. (1) and (2) hold for arbitrarily shaped domains with applied external traction. Our algorithm specializes to polygonal domains, circular holes, and piecewise constant traction. The reasons for restricting our focus are that these
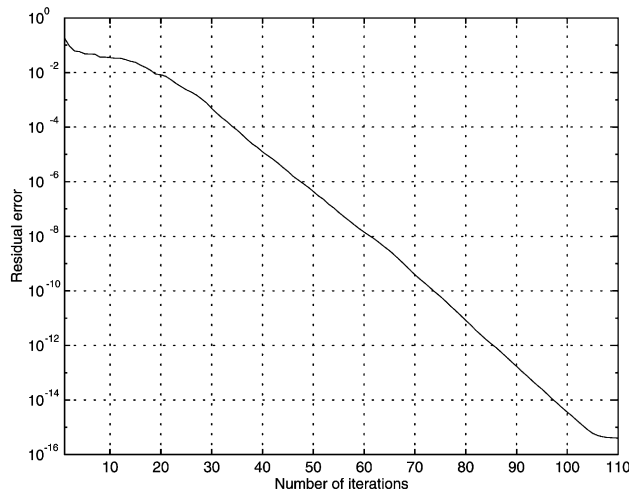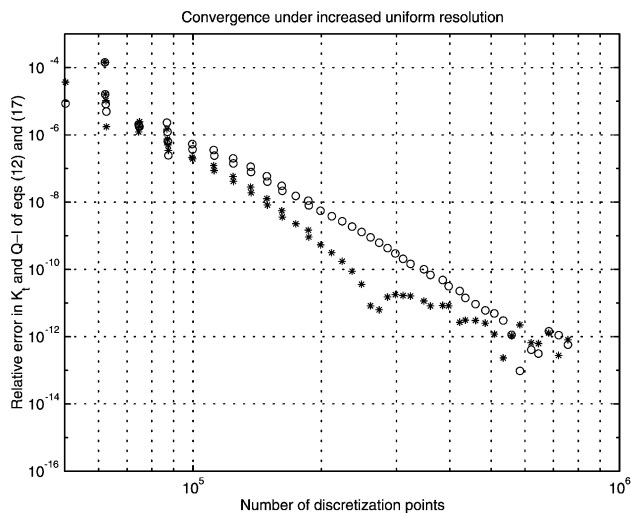

Fig. 11. Left: convergence of the notch stress intensity factor Q_I of Eq. (17), shown as stars, and the stress concentration factor K_t of Eq. (12), shown as open circles, for the setup in Fig. 10. Reference values are taken as $Q_I = 11.02821540165$ and $K_t = 55.06854533610$. Right: a GMRES iteration history for a computation with $N = 756,938$ discretization points.

geometries are common in the engineering literature and that we want to establish benchmarks for simply reproducible setups. A more general solver would require more work, as we now outline: allowing for non-circular holes and for non-constant tractions is simple. The circular property of the holes is used in the analytic evaluation of the rightmost integral of Eq. (19). This integral only involves known geometric quantities and it can be evaluated numerically almost without precision loss for holes of arbitrary shapes. The situation is similar for the piecewise constant property of the applied traction, which is used for analytic evaluation of the right-hand sides of Eqs. (1) and (2). In Ref. [27] we have successfully tested a numerical implementation of the operator M_1 in this context. Allowing for curved corner boundaries is harder. Far away from corners, no assumption about the shape of the boundary is made. Close to, or on, corner panels, however, we use the property that the corner legs are straight—both in the analysis of the stress field leading up to the expansion (14) and in the evaluation of the action on the operator M_3 on the basis functions in this series. The expansion (14) will hold, asymptotically, also for curved corner boundaries, but the extraction of the expansion coefficients will become more difficult in the absence of symmetry. Furthermore, the integrals corresponding to those in Eqs. (20) and (21) will be more involved and we can make less use of precomputation when corner panels of different sizes are no longer congruent. These modifications are likely to degrade the performance of the code and may force us to lower the order of the scheme. Yet another extension is to allow for displacement boundary conditions. This requires the replacement of the Eqs. (1) and (2), based on the potentials $\Phi(z)$ and $\Psi(z)$, with equally efficient Fredholm equations for the displacement problem. This problem is open and its solution could involve an analysis resembling that of Ref. [29] but based on the potentials $\phi(z)$ and $\psi(z)$, which are primitive functions of $\Phi(z)$ and $\Psi(z)$. See also Sherman [47].

Disregarding the present restrictions in applicability, our algorithm is still not entirely ideal. Its performance degrades for large included vertex angles. The use of a Galerkin scheme, instead of a Nyström scheme, could, perhaps, eliminate some ill-conditioning related to the change of representation on corner panels. Another worry is that a number of parameters related to the quadrature on the corner panels are determined in an ad hoc manner. More optimal, and theoretically derived, choices could make the scheme even more efficient. If an effort in this direction is worthwhile or not depends on the need for better stability and on the progress of alternative approaches. Work by Yarvin and Rokhlin, involving generalized Gaussian quadrature and singular value decomposition of integral operators on corners, is reported to be in preparation [53]. A more radical way to go is to abandon the idea of solving integral equations completely, and instead compute the solution to the stress problem directly, as a sum of reflected layer potentials. Promising work on this type of fast direct solvers have been reported by Ethridge and Greengard for Laplace's and Poisson's equations on the unit square, and extensions to complex geometry are underway [15].

We have avoided giving explicit timings for our computations. The reason is that we focused more on testing the stability of our scheme than on achieving the fastest possible execution. All tolerances, for example in the fast multipole method and in the GMRES iterative solver, have been set to approximately machine epsilon, corresponding to $p = 53$ terms in multipole and local Taylor expansions. Compensated summation has been used in all direct matrix–vector multiplications. As for the large-scale example of Section 5.5, we can say the following: virtually, all computational work takes place in the fast multipole subroutine. The largest computation that we undertook, with 756,938 discretization points and 110 iterations in Section 5.5, required approximately 18 h of computing time on a SunBlade 100 workstation using the compiler option (-O4).

Acknowledgements

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