



## Models of corner and crack singularity of linear elastostatics and their numerical solutions

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### ABSTRACT

The singular solutions for linear elastostatics at corners are *essential* in both theory and computation. In this paper we seek the singular solutions for corners with the clamped and the free stress boundary conditions, and explore corner singularity in detail. In this paper the singular solutions of linear elastostatics are derived, and two new models of interior crack singularity are proposed. The collocation Trefftz methods are used to obtain highly accurate solutions, where the leading coefficient has 14 (or 12) significant digits by the computation with double precision. Such solutions are useful to examine other numerical methods for singularity problems in linear elastostatics. Also the explicit singular solutions can be adapted to design and develop efficient numerical methods for singularity problems, such as the combined method (Li, 1998, 2008 [19,20]) and the Trefftz methods which include the boundary approximation method (Li, 1990, Li et al., 1987 [18,26]), the collocation Trefftz method (Li et al., 2008 [24]), the hybrid Trefftz method (Qin, 2000 [36]), the boundary collocation techniques (Kolodziej and Zielinski, 2009 [16]), etc. This paper also explores a systematic analysis for singularity properties and explicit singular solutions for corners of linear elastostatics.

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### 1. Introduction

The traditional finite element method (FEM) and finite difference method (FDM) provide poor accuracy of numerical solutions for singularity problems. Many innovative methods have been developed, to seek the numerical solutions with optimal convergence rates and good stability. For Poisson's equation and other elliptic equations, a systematic study is provided in Li [19]. The singular properties and the singular solutions near the corners are crucial to design effective numerical methods. Moreover, highly accurate solutions are important to examine different numerical methods. For Laplace's equation, Motz's problem is a benchmark of singularity problems, and its highly accurate solutions are provided in [19,23,26,30] by the collocation Trefftz method (CTM).

For biharmonic equation, similar models to Motz's problem are first developed in [23], and stability analysis is explored in [25]. The singular solutions near corners under free stress boundary conditions were first given in Williams [38], and then in Lin and Tong [28], Jirousek and Venkatesh [14], Jirousek and Wroblewski [15], Piltner [32], Drombosky et al. [8], and Qin [36]. In this paper,

our efforts are paid to linear elastostatics, and to derive new particular solutions of linear elastostatics near corners under the clamped, and the free stress boundary conditions, in addition two new crack models are proposed. It is worthy pointing out that the singular solutions near corners in this paper directly from free stress boundary conditions are coincident with those in [38,28] from biharmonic equations by using a similarity mapping.

The singular properties and the explicit singular solutions of linear elastostatics at corners are *essential* in both theory and computation. Once the singularity of corner solutions is known, the reduced convergence rates of FEM, FDM, FVM and other numerical methods are found (see Section 7.2), and some improved techniques can be invented, to recover the optimal convergence rates (see [19]). Moreover, once the explicit singular solutions are known, some models as in Sections 4 and 6 can be designed, and the collocation Trefftz method can be used to give their highly accurate solutions, which can be used for testing other numerical methods (also see [19]). More importantly, based on the explicit particular solutions of corners given in this paper, we may develop a number of efficient numerical methods for linear elastostatics, such as the Trefftz methods including the collocation Trefftz method and the hybrid Trefftz method, and the combinations (see [1,4,7–9,12,17,20,27,29,34,35,39].)

This paper is organized as follows. In Section 2, a basic description for elastostatics problems in 2D is introduced, and their particular solutions are provided. In Section 3, singular

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solutions near corners are derived for the clamped boundary conditions. In Section 4, a model of crack singularity with clamped boundary conditions is designed, and its numerical solutions are sought by the collocation Trefftz method (CTM) as in [19,24]. In Section 5, singular solutions near corners are derived for the free stress boundary conditions, and in Section 6 the other model of crack singularity with free stress is designed, and numerical results are also provided. In Section 7, the algorithms are given for the leading powers  $v_k$  of the corner solution  $O(r^{v_k})$ , and numerical results are provided for the rectangular corner and the concave corner of the L-shaped domain. In the last section, a few concluding remarks are made.

## 2. Linear elastostatics problems in 2D

### 2.1. Basic equations

Consider the linear elastostatics problem in 2D. Denote the displacement vector,

$$\vec{w} = \mathbf{w} = \{w_1(\mathbf{x}), w_2(\mathbf{x})\}^T = \{u(x, y), v(x, y)\}^T, \quad (2.1)$$

where  $\vec{x} = \mathbf{x} = (x_1, x_2) = (x, y)$ . The linear strain tensor is given by

$$\varepsilon_{ij}(\mathbf{x}) = \frac{1}{2} \left[ \frac{\partial w_i(\mathbf{x})}{\partial x_j} + \frac{\partial w_j(\mathbf{x})}{\partial x_i} \right], \quad 1 \leq i, j \leq 2. \quad (2.2)$$

Let  $\sigma_{ij}$  ( $1 \leq i, j \leq 2$ ) denote the stress tensor at  $\mathbf{x}$ . For an isotropic homogeneous Hookean solid, there exist the stress-strain relations

$$\sigma_{ij} = \lambda(\nabla \cdot \vec{w})\delta_{ij} + 2\mu\varepsilon_{ij}, \quad 1 \leq i, j \leq 2, \quad (2.3)$$

where “ $\nabla \cdot$ ” is the divergence operator,  $\lambda$  and  $\mu$  are the Lamé constants, and  $\delta_{ij}$  is the Kronecker delta.

When there exists a body force  $\vec{f}$ , we obtain the nonhomogeneous equation, called the Lamé system for isotropic body:

$$\mu\Delta\vec{w} + (\lambda + \mu)\nabla(\nabla \cdot \vec{w}) + \vec{f} = 0 \quad \text{in } S. \quad (2.4)$$

When  $\vec{f} \equiv \vec{0}$ , we have the Cauchy-Navier equation of linear elastostatics:

$$\Delta\vec{w} + \frac{1}{1-2\nu}\nabla(\nabla \cdot \vec{w}) = 0 \quad \text{in } S, \quad (2.5)$$

where the Poisson ratio

$$\nu = \frac{\lambda}{2(\lambda + \mu)}, \quad 0 < \nu < \frac{1}{2}. \quad (2.6)$$

Young’s modulus  $E$  and the bulk modulus  $K$  are introduced by

$$E = \frac{\mu(3\lambda + 2\mu)}{\lambda + \mu}, \quad K = \frac{E}{3(1-2\nu)}. \quad (2.7)$$

The inverse relations of (2.6) and (2.7) are given by

$$\lambda = \frac{Ev}{(1+\nu)(1-2\nu)}, \quad \mu = \frac{E}{2(1+\nu)}. \quad (2.8)$$

The strain-stress relations are given by

$$\varepsilon_{ij} = \frac{1+\nu}{E}\sigma_{ij} - \frac{\nu}{E}\delta_{ij}\sum_{k=1}^2\sigma_{kk}. \quad (2.9)$$

There also exist the symmetric relations:

$$\sigma_{ij} = \sigma_{ji}, \quad \varepsilon_{ij} = \varepsilon_{ji}. \quad (2.10)$$

Denote the constant

$$\kappa = \frac{1}{4(1-\nu)}. \quad (2.11)$$

For the plane strain problem the constant is given by

$$D = \frac{\lambda + \mu}{\lambda + 3\mu} = \frac{1}{3-4\nu} = \frac{\kappa}{1-\kappa}, \quad (2.12)$$

and for the plane stress problem,

$$D = \frac{1}{3-4\nu} = \frac{1+\hat{\nu}}{3-\hat{\nu}}, \quad \nu = \frac{\hat{\nu}}{1+\hat{\nu}}. \quad (2.13)$$

### 2.2. Traction boundary conditions

The Cauchy-Navier equation (2.5) is written explicitly by

$$\mu\Delta u + (\lambda + \mu)\left\{\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 v}{\partial x\partial y}\right\} = 0 \quad \text{in } S, \quad (2.14)$$

$$\mu\Delta v + (\lambda + \mu)\left\{\frac{\partial^2 u}{\partial x\partial y} + \frac{\partial^2 v}{\partial y^2}\right\} = 0 \quad \text{in } S, \quad (2.15)$$

or by

$$\Delta u + \frac{1}{1-2\nu}\left\{\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 v}{\partial x\partial y}\right\} = 0 \quad \text{in } S, \quad (2.16)$$

$$\Delta v + \frac{1}{1-2\nu}\left\{\frac{\partial^2 u}{\partial x\partial y} + \frac{\partial^2 v}{\partial y^2}\right\} = 0 \quad \text{in } S, \quad (2.17)$$

where  $\nu$  is given in (2.6). The traction on  $\partial S$  is given by

$$\vec{t}(\vec{w})(\mathbf{x}) = (\tau_1(u, v), \tau_2(u, v))^T, \quad (2.18)$$

where the components

$$\tau_1(u, v) = \sigma_x n_1 + \sigma_{xy} n_2 = \lambda\left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y}\right)n_1 + 2\mu\frac{\partial u}{\partial v} + \mu n_2\left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y}\right), \quad (2.19)$$

$$\tau_2(u, v) = \sigma_{xy} n_1 + \sigma_y n_2 = \lambda\left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y}\right)n_2 + 2\mu\frac{\partial v}{\partial v} - \mu n_1\left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y}\right), \quad (2.20)$$

where  $n_1 = \cos(\nu, x)$ ,  $n_2 = \cos(\nu, y)$ , and the stress

$$\sigma_x = \sigma_{11}, \quad \sigma_y = \sigma_{22}, \quad \sigma_{xy} = \sigma_{12} = \sigma_{21}. \quad (2.21)$$

### 2.3. Particular solutions

In Jirousek and Wroblewski [15], Jirousek and Venkstesh [14] and Qin [36], for the plane stress equations (2.16) and (2.17), the particular solutions are expressed as the complex functions. Denote  $i = \sqrt{-1}$ ,  $\mathbf{z} = x + iy = re^{i\theta}$ ,  $\bar{\mathbf{z}} = x - iy = re^{-i\theta}$ . The particular solutions  $u(x, y)$  and  $v(x, y)$  of the plane stress equations are given by the real and imaginary parts of  $A_k, B_k, C_k$  and  $D_k$  below, respectively,

$$A_k = iz^k + iDkz\bar{z}^{k-1}, \quad (2.22)$$

$$B_k = z^k - Dkz\bar{z}^{k-1}, \quad (2.23)$$

$$C_k = i\bar{z}^k, \quad (2.24)$$

$$D_k = -\bar{z}^k, \quad k = 1, 2, \dots, \quad (2.25)$$

where  $D$  is given in (2.12) or (2.13). We have the following linear combination for the Trefftz method (TM):

$$u_L = \sum_{k=1}^L \{a_k \Re(A_k) + b_k \Re(B_k) + c_k \Re(C_k) + d_k \Re(D_k)\} + d_0, \quad (2.26)$$

$$v_L = \sum_{k=1}^L \{a_k \Im(A_k) + b_k \Im(B_k) + c_k \Im(C_k) + d_k \Im(D_k)\} + c_0, \quad (2.27)$$

where  $a_k, b_k, c_k, d_k, c_0$  and  $d_0$  are the constants, and the notations  $\Re$  and  $\Im$  are the real and the imaginary parts, respectively. In (2.26) and (2.27)  $u = \Re(A_1) = -(1+D)y$  and  $v = \Im(A_1) = (1+D)x$  denote a rigid motion.

From (2.26) and (2.27) we have the complete particular solutions (also see [15,36]),

$$u_L = \sum_{k=1}^L r^k \{a_k[-\sin k\theta + Dk\sin(k-2)\theta] + b_k[\cos k\theta - Dk\cos(k-2)\theta] + c_k \sin k\theta - d_k \cos k\theta\} + d_0, \tag{2.28}$$

$$v_L = \sum_{k=1}^L r^k \{a_k[\cos k\theta + Dk\cos(k-2)\theta] + b_k[\sin k\theta + Dk\sin(k-2)\theta] + c_k \cos k\theta + d_k \sin k\theta\} + c_0. \tag{2.29}$$

### 3. Singular solutions near corners under clamped boundary conditions

Choose the sectorial domain  $S = \{(r, \theta) | (0 \leq R, -\theta \leq \theta \leq \Theta)\}$ , where  $\Theta \in (0, \pi]$ . First consider the displacement conditions at two edges of corner  $O$  with the clamped boundary conditions

$$u = v = 0 \quad \text{on } \theta = \pm \Theta. \tag{3.1}$$

The singularity solutions can be obtained from (2.22) to (2.25) by replacing the integer  $k$  with a complex number  $v^* = v_k = \alpha_k + i\beta_k$ <sup>1</sup>, where  $\alpha_k$  and  $\beta_k$  are real and  $i = \sqrt{-1}$ . Then the general singular solutions are given from (2.28) and (2.29)

$$u_L = \sum_{k=1}^L r^{v_k} \{a_k[-\sin v_k\theta + Dv_k\sin(v_k-2)\theta] + b_k[\cos v_k\theta - Dv_k\cos(v_k-2)\theta] + c_k \sin v_k\theta - d_k \cos v_k\theta\} + d_0, \tag{3.2}$$

$$v_L = \sum_{k=1}^L r^{v_k} \{a_k[\cos v_k\theta + Dv_k\cos(v_k-2)\theta] + b_k[\sin v_k\theta + Dv_k\sin(v_k-2)\theta] + c_k \cos v_k\theta + d_k \sin v_k\theta\} + c_0, \tag{3.3}$$

where  $a_k, b_k, c_k$  and  $d_k$  are real coefficients, and  $D$  is given in (2.12) or (2.13). In (3.2) and (3.3), we choose a group of four coefficients  $a_k, b_k, c_k$  and  $d_k$  to satisfy the boundary conditions in (3.1), to obtain  $d_0 = c_0 = 0$  and the matrix equation

$$\mathbf{B}\mathbf{y} = \mathbf{0}, \tag{3.4}$$

where  $\mathbf{y} = (a, b, c, d)^T$  and the matrix is given by

$$\mathbf{B} = \begin{pmatrix} -\sin v^*\theta + Dv^*\sin(v^*-2)\theta & \cos v^*\theta - Dv^*\cos(v^*-2)\theta & \sin v^*\theta & -\cos v^*\theta \\ \cos v^*\theta + Dv^*\cos(v^*-2)\theta & \sin v^*\theta + Dv^*\sin(v^*-2)\theta & \cos v^*\theta & \sin v^*\theta \\ \sin v^*\theta - Dv^*\sin(v^*-2)\theta & \cos v^*\theta - Dv^*\cos(v^*-2)\theta & -\sin v^*\theta & -\cos v^*\theta \\ \cos v^*\theta + Dv^*\cos(v^*-2)\theta & -\sin v^*\theta - Dv^*\sin(v^*-2)\theta & \cos v^*\theta & -\sin v^*\theta \end{pmatrix}. \tag{3.5}$$

The sufficient and necessary condition of existence of nonzero solutions  $\mathbf{y}$  is that the determinant of matrix  $\mathbf{B}$  is zero,

$$\begin{vmatrix} -\sin v^*\theta + Dv^*\sin(v^*-2)\theta & \cos v^*\theta - Dv^*\cos(v^*-2)\theta & \sin v^*\theta & -\cos v^*\theta \\ \cos v^*\theta + Dv^*\cos(v^*-2)\theta & \sin v^*\theta + Dv^*\sin(v^*-2)\theta & \cos v^*\theta & \sin v^*\theta \\ \sin v^*\theta - Dv^*\sin(v^*-2)\theta & \cos v^*\theta - Dv^*\cos(v^*-2)\theta & -\sin v^*\theta & -\cos v^*\theta \\ \cos v^*\theta + Dv^*\cos(v^*-2)\theta & -\sin v^*\theta - Dv^*\sin(v^*-2)\theta & \cos v^*\theta & -\sin v^*\theta \end{vmatrix} = 0. \tag{3.6}$$

We have a lemma.

**Lemma 3.1.** For nonzero solutions of the equations

$$\begin{pmatrix} b_{11} & b_{12} & b_{13} & b_{14} \\ b_{21} & b_{22} & b_{23} & b_{24} \\ -b_{11} & b_{12} & -b_{13} & b_{14} \\ b_{21} & -b_{22} & b_{23} & -b_{24} \end{pmatrix} \begin{pmatrix} a \\ b \\ c \\ d \end{pmatrix} = \vec{0}, \tag{3.7}$$

there exist the equalities

$$\begin{vmatrix} b_{11} & b_{13} \\ b_{21} & b_{23} \end{vmatrix} = 0, \quad \begin{vmatrix} b_{12} & b_{14} \\ b_{22} & b_{24} \end{vmatrix} = 0. \tag{3.8}$$

**Proof.** We may split the equations in (3.7) as follows. In (3.7), by subtracting the third equation from the first equation, and by adding the second and the fourth equations, we have

$$2b_{11}a + 2b_{13}c = 0, \quad 2b_{21}a + 2b_{23}c = 0. \tag{3.9}$$

The existence of nonzero solutions of  $a$  and  $c$  implies

$$\begin{vmatrix} b_{11} & b_{13} \\ b_{21} & b_{23} \end{vmatrix} = 0. \tag{3.10}$$

Similarly, in (3.7) by adding the first and the third equations, and by subtracting the fourth from the second equations, we have

$$2b_{12}b + 2b_{14}d = 0, \quad 2b_{22}b + 2b_{24}d = 0. \tag{3.11}$$

The existence of nonzero solutions of  $b$  and  $d$  implies

$$\begin{vmatrix} b_{12} & b_{14} \\ b_{22} & b_{24} \end{vmatrix} = 0. \tag{3.12}$$

This completes the proof of Lemma 3.1.  $\square$

**Theorem 3.1.** Let (3.6) be given for the clamped boundary conditions (3.1). There exist the equalities

$$\sin 2v^*\theta = \pm Dv^*\sin 2\theta, \tag{3.13}$$

where the signs “+” and “-” correspond to the symmetric and the anti-symmetric solution  $u, v$ , respectively.

**Proof.** Based on Lemma 3.1, we have from (3.6)

$$\begin{vmatrix} -\sin v^*\theta + Dv^*\sin(v^*-2)\theta & \sin v^*\theta \\ \cos v^*\theta + Dv^*\cos(v^*-2)\theta & \cos v^*\theta \end{vmatrix} = 0 \tag{3.14}$$

and

$$\begin{vmatrix} \cos v^*\theta - Dv^*\cos(v^*-2)\theta & -\cos v^*\theta \\ \sin v^*\theta + Dv^*\sin(v^*-2)\theta & \sin v^*\theta \end{vmatrix} = 0. \tag{3.15}$$

From (3.14) there is the equality,

$$(-\sin v^*\theta + Dv^*\sin(v^*-2)\theta)\cos v^*\theta = (\cos v^*\theta + Dv^*\cos(v^*-2)\theta)\sin v^*\theta.$$

Since

$$\sin(v^*-2)\theta = \sin v^*\theta \cos 2\theta - \cos v^*\theta \sin 2\theta, \tag{3.16}$$

$$\cos(v^*-2)\theta = \cos v^*\theta \cos 2\theta + \sin v^*\theta \sin 2\theta, \tag{3.17}$$

we have

$$2\sin v^*\theta \cos v^*\theta = -Dv^*(\cos^2 v^*\theta + \sin^2 v^*\theta)\sin 2\theta, \tag{3.18}$$

that is,

$$\sin 2v^*\theta = -Dv^*\sin 2\theta. \tag{3.19}$$

This is the desired result (3.13) with sign “-”.

Similarly, we have from (3.15)

$$(\cos v^*\theta - Dv^*\cos(v^*-2)\theta)\sin v^*\theta = -(\sin v^*\theta + Dv^*\sin(v^*-2)\theta)\cos v^*\theta.$$

From (3.16) and (3.17) we have

$$\sin 2v^*\theta = Dv^*\sin 2\theta. \tag{3.20}$$

<sup>1</sup> In this paper, the complex numbers  $v^*$  and  $v_k$  are used, which are different from the Poisson ratio  $\nu$  in (2.6).

Combining (3.19) and (3.20) yields the desired results (3.13) with sign “ ± ”.

The equality (3.13) with sign “ + ” results from (3.15), which denotes the nonzero coefficients  $b_k$  and  $d_k$  of symmetric solutions  $u, v$  in (3.2) and (3.3) satisfying  $\partial u/\partial y = v = 0$  at  $\theta = 0$ . Also the equality (3.13) with the sign “ - ” responds to the nonzero  $a_k$  and  $c_k$  of the anti-symmetric solutions  $u, v$  satisfying  $u = \partial v/\partial y = 0$  at  $\theta = 0$ . This completes the proof of Theorem 3.1. □

When the real or the complex root  $v_k$  are obtained from (3.13), the coefficients satisfy the following relations:

$$c_k = p_k a_k, \quad d_k = q_k b_k, \tag{3.21}$$

where  $p_k$  and  $q_k$  may be complex, given by

$$p_k = -\{Dv_k \cos 2\theta + \sqrt{1 - (Dv_k \sin 2\theta)^2}\}, \tag{3.22}$$

$$q_k = -Dv_k \cos 2\theta + \sqrt{1 - (Dv_k \sin 2\theta)^2}. \tag{3.23}$$

Then the particular solutions of (3.2) and (3.3) are simplified as

$$u_L = \sum_{k=1}^L \Re\{r^{v_k} \{a_k [(-1 + p_k) \sin v_k \theta + Dv_k \sin(v_k - 2)\theta] + b_k [(1 - q_k) \cos v_k \theta - Dv_k \cos(v_k - 2)\theta]\}, \tag{3.24}$$

$$v_L = \sum_{k=1}^L \Re\{r^{v_k} \{a_k [(1 + p_k) \cos v_k \theta + Dv_k \cos(v_k - 2)\theta] + b_k [(1 + q_k) \sin v_k \theta + Dv_k \sin(v_k - 2)\theta]\}, \tag{3.25}$$

where  $a_k$  and  $b_k$  are still real.

Below, let us derive the constants  $p_k$  and  $q_k$  in (3.22) and (3.23). In general, we assume

$$\cos v^* \theta \neq 0, \quad \sin v^* \theta \neq 0. \tag{3.26}$$

Otherwise, when  $\cos v^* \theta = 0$  (i.e.,  $v^* \theta = (n + \frac{1}{2})\pi$ ) and  $\sin v^* \theta = 0$  (i.e.,  $v^* \theta = n\pi$ ), the coefficient relations are given in the next section. The relation between coefficients  $a$  and  $c$  can be found in (3.14) under (3.19)

$$c = \frac{1}{\sin v^* \theta} (\sin v^* \theta - Dv^* \sin(v^* - 2)\theta) a, \tag{3.27}$$

to give

$$\begin{aligned} p(v^*) &= \frac{1}{\sin v^* \theta} (\sin v^* \theta - Dv^* \sin(v^* - 2)\theta) \\ &= 1 - Dv^* \frac{1}{\sin v^* \theta} (\sin v^* \theta \cos 2\theta - \cos v^* \theta \sin 2\theta) \\ &= 1 - Dv^* \cos 2\theta + Dv^* \frac{1}{\sin v^* \theta} \cos v^* \theta \sin 2\theta \\ &= 1 - Dv^* \cos 2\theta + Dv^* \frac{1}{\sin v^* \theta} \cos v^* \theta \left[ -\frac{\sin 2v^* \theta}{Dv^*} \right] \\ &= 1 - Dv^* \cos 2\theta - 2(\cos v^* \theta)^2, \end{aligned} \tag{3.28}$$

where we have used (3.19). Since  $2 \cos^2 x = 1 + \cos 2x$  we have from (3.28)

$$\begin{aligned} p(v^*) &= 1 - Dv^* \cos 2\theta - 1 - \cos 2v^* \theta \\ &= -Dv^* \cos 2\theta - \sqrt{1 - (\sin 2v^* \theta)^2} \\ &= -\{Dv^* \cos 2\theta + \sqrt{1 - (Dv^* \sin 2\theta)^2}\}. \end{aligned} \tag{3.29}$$

This is the first desired result (3.22) with  $v^* = v_k$ .

Next for coefficients  $b$  and  $d$ , we have from (3.15) under (3.20),

$$d = \frac{1}{\cos v^* \theta} (\cos v^* \theta - Dv^* \cos(v^* - 2)\theta) b = q(v^*) b, \tag{3.30}$$

to give

$$\begin{aligned} q(v^*) &= \frac{1}{\cos v^* \theta} (\cos v^* \theta - Dv^* \cos(v^* - 2)\theta) \\ &= 1 - Dv^* \frac{1}{\cos v^* \theta} (\cos v^* \theta \cos 2\theta + \sin v^* \theta \sin 2\theta) \\ &= 1 - Dv^* \cos 2\theta - Dv^* \frac{1}{\cos v^* \theta} \sin v^* \theta \sin 2\theta \\ &= 1 - Dv^* \cos 2\theta - Dv^* \frac{1}{\cos v^* \theta} \sin v^* \theta \left[ \frac{\sin 2v^* \theta}{Dv^*} \right] \\ &= 1 - Dv^* \cos 2\theta - 2(\sin v^* \theta)^2. \end{aligned} \tag{3.31}$$

Since  $2 \sin^2 x = 1 - \cos 2x$  we have

$$\begin{aligned} q(v^*) &= 1 - Dv^* \cos 2\theta - 1 + \cos 2v^* \theta \\ &= -Dv^* \cos 2\theta + \sqrt{1 - (\sin 2v^* \theta)^2} \\ &= -Dv^* \cos 2\theta + \sqrt{1 - (Dv^* \sin 2\theta)^2}. \end{aligned} \tag{3.32}$$

This is the second desired result (3.23).

It is interesting to find out the solutions  $v^*$  at  $\theta = k\pi/4$  with  $k = 1, 2, 3, 4$ .  $\theta = \pi/4$  and  $3\pi/4$  respond to the right angle and the L-shaped domains, and the numerical  $v_k$  based on Theorem 3.1 are provided in Section 7.

When  $\theta = \pi/2, \pi$ , we may obtain the real roots explicitly. When  $\theta = \pi/2$ , we have  $\sin \alpha\pi = 0$  from Theorem 3.1, and then  $v^* = k$  to indicate that there is no singularity. When  $\theta = \pi$  denotes an interior crack within the plane, which is very important and interesting in both theory and computation. We have from Theorem 3.1 for  $\beta = 0$ , to yield

$$\sin 2\alpha\pi = \pm D\alpha \sin 2\pi = 0, \tag{3.33}$$

to give

$$\alpha = \frac{n}{2}, \quad n = 1, 2, \dots \tag{3.34}$$

Since  $\theta = \pi$ , Eqs. (3.26) are violated, the particular solutions cannot be obtained directly from (3.24) and (3.25). We may derive the simplified singular solutions directly. From (3.4) we have the boundary conditions at  $\theta = \pi$

$$\begin{pmatrix} -\sin v^* \pi + Dv^* \sin(v^* - 2)\pi & \cos v^* \pi - Dv^* \cos(v^* - 2)\pi & \sin v^* \pi & -\cos v^* \pi \\ \cos v^* \pi + Dv^* \cos(v^* - 2)\pi & \sin v^* \pi + Dv^* \sin(v^* - 2)\pi & \cos v^* \pi & \sin v^* \pi \\ \sin v^* \pi - Dv^* \sin(v^* - 2)\pi & \cos v^* \pi - Dv^* \cos(v^* - 2)\pi & -\sin v^* \pi & -\cos v^* \pi \\ \cos v^* \pi + Dv^* \cos(v^* - 2)\pi & -\sin v^* \pi - Dv^* \sin(v^* - 2)\pi & \cos v^* \pi & -\sin v^* \pi \end{pmatrix} \begin{pmatrix} a \\ b \\ c \\ d \end{pmatrix} = \begin{pmatrix} (-1 + Dv^*) \sin v^* \pi & (1 - Dv^*) \cos v^* \pi & \sin v^* \pi & -\cos v^* \pi \\ (1 + Dv^*) \cos v^* \pi & (1 + Dv^*) \sin v^* \pi & \cos v^* \pi & \sin v^* \pi \\ -(-1 + Dv^*) \sin v^* \pi & (1 - Dv^*) \cos v^* \pi & -\sin v^* \pi & -\cos v^* \pi \\ (1 + Dv^*) \cos v^* \pi & -(1 + Dv^*) \sin v^* \pi & \cos v^* \pi & -\sin v^* \pi \end{pmatrix} \begin{pmatrix} a \\ b \\ c \\ d \end{pmatrix} = \vec{0}. \tag{3.35}$$

From (3.34), we may consider two cases of  $v^* = n + \frac{1}{2}$  and  $v^* = n$ , respectively. First for  $v^* = n + \frac{1}{2}$  we have from (3.35)

$$\begin{pmatrix} (-1 + Dv^*) \sin(n + \frac{1}{2})\pi & 0 & \sin(n + \frac{1}{2})\pi & 0 \\ 0 & (1 + Dv^*) \sin(n + \frac{1}{2})\pi & 0 & \sin(n + \frac{1}{2})\pi \\ (1 - Dv^*) \sin(n + \frac{1}{2})\pi & 0 & -\sin(n + \frac{1}{2})\pi & 0 \\ 0 & -(1 + Dv^*) \sin(n + \frac{1}{2})\pi & 0 & -\sin(n + \frac{1}{2})\pi \end{pmatrix} \times \begin{pmatrix} a \\ b \\ c \\ d \end{pmatrix} = \vec{0}, \tag{3.36}$$

to give the relations of coefficients

$$c = (1 - D(n + \frac{1}{2}))a, \quad d = -(1 + D(n + \frac{1}{2}))b \text{ for } v^* = n + \frac{1}{2}. \tag{3.37}$$

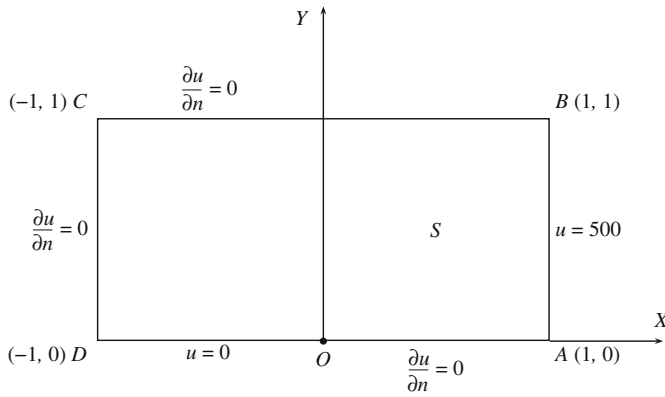


Fig. 1. Motz's problem.

Next for  $v^* = n$  we have from (3.35)

$$\begin{pmatrix} 0 & (1-Dn)\cos n\pi & 0 & -\cos n\pi \\ (1+Dn)\cos n\pi & 0 & \cos n\pi & 0 \\ 0 & (1-Dn)\cos n\pi & 0 & -\cos n\pi \\ (1+Dn)\cos n\pi & 0 & \cos n\pi & 0 \end{pmatrix} \begin{pmatrix} a \\ b \\ c \\ d \end{pmatrix} = \vec{0}, \quad (3.38)$$

to give

$$c = -(1+Dn)a, \quad d = (1-Dn)b \text{ for } v^* = n. \quad (3.39)$$

By applying (3.37) and (3.39), the particular solutions in (3.2) and (3.3) are simplified as

$$\begin{aligned} u_L = & \sum_{n=0}^{L-1} r^{n+1/2} \left\{ a_{2n+1} D \left( n + \frac{1}{2} \right) \left[ -\sin \left( n + \frac{1}{2} \right) \theta + \sin \left( n - \frac{3}{2} \right) \theta \right] \right. \\ & + b_{2n+1} \left[ \left( 2 + D \left( n + \frac{1}{2} \right) \right) \cos \left( n + \frac{1}{2} \right) \theta \right. \\ & \left. \left. - D \left( n + \frac{1}{2} \right) \cos \left( n - \frac{3}{2} \right) \theta \right] \right\} \\ & + \sum_{n=1}^L r^n \{ a_{2n} [-(2+Dn)\sin n\theta + Dn\sin(n-2)\theta] \\ & + b_{2n} Dn [\cos n\theta - \cos(n-2)\theta] \}, \end{aligned} \quad (3.40)$$

$$\begin{aligned} v_L = & \sum_{n=0}^{L-1} r^{n+1/2} \left\{ a_{2n+1} \left[ \left( 2 - D \left( n + \frac{1}{2} \right) \right) \cos \left( n + \frac{1}{2} \right) \theta \right. \right. \\ & + D \left( n + \frac{1}{2} \right) \cos \left( n - \frac{3}{2} \right) \theta \left. \right] + b_{2n+1} D \left( n + \frac{1}{2} \right) \left[ -\sin \left( n + \frac{1}{2} \right) \theta \right. \\ & \left. + \sin \left( n - \frac{3}{2} \right) \theta \right] \right\} + \sum_{n=1}^L r^n \{ a_{2n} Dn [-\cos n\theta + \cos(n-2)\theta] \\ & + b_{2n} [(2-Dn)\sin n\theta + Dn\sin(n-2)\theta] \}. \end{aligned} \quad (3.41)$$

#### 4. Model A of crack singularity and its numerical solutions

##### 4.1. Model A of crack singularity

Let us mimic Motz's problem of the Laplace equation satisfying the boundary conditions in Fig. 1 [23] for elastostatics. For the rectangle  $S = \{(x, y) | -1 \leq x \leq 1, 0 \leq y \leq 1\}$ , there exists a crack line  $\overline{OD}$  under the clamped boundary conditions, and the symmetric boundary conditions on  $\overline{OA} \cup \overline{BC} \cup \overline{CD}$ . On  $\overline{AB}$  we suppose that  $u = 1$  and no exterior force along the  $y$  direction, which implies

$\sigma_{xy} = u_y + v_x = 0$ .<sup>2</sup> Since  $u_y = 0$  on  $\overline{AB}$ , the condition  $v_x = 0$  is obtained. Then the Model A is designed as the Cauchy-Navier equation (2.5) satisfying the following boundary conditions:

$$u = v = 0 \text{ on } \overline{OD}, \quad (4.1)$$

$$v = u_y = 0 \text{ on } \overline{OA} \cup \overline{CB}, \quad (4.2)$$

$$u = v_x = 0 \text{ on } \overline{CD}, \quad (4.3)$$

$$u = 1, \quad v_x = 0 \text{ on } \overline{AB}. \quad (4.4)$$

To satisfy the boundary condition on  $\overline{OA}$  with  $\theta = 0$ , for solution  $u$  in (3.40) we have from  $u_y = \partial u / r \partial \theta = 0$ , i.e.,

$$a_{2n+1} D \left( n + \frac{1}{2} \right) \left[ - \left( n + \frac{1}{2} \right) + \left( n - \frac{3}{2} \right) \right] = -a_{2n+1} D (2n + 1) = 0,$$

$$a_{2n} [-(2+Dn)n + Dn(n-2)] = -2n(D+1)a_{2n} + 1 = 0. \quad (4.5)$$

Then we conclude  $a_{2n+1} = a_{2n} = 0$ . Eqs. (3.40) and (3.41) lead to

$$\begin{aligned} u_L = & \sum_{n=0}^{L-1} b_{2n+1} r^{n+1/2} \left[ \left( 2 + D \left( n + \frac{1}{2} \right) \right) \cos \left( n + \frac{1}{2} \right) \theta \right. \\ & \left. - D \left( n + \frac{1}{2} \right) \cos \left( n - \frac{3}{2} \right) \theta \right] + \sum_{n=1}^L b_{2n} r^n \{ Dn [\cos n\theta - \cos(n-2)\theta] \}, \end{aligned} \quad (4.6)$$

$$\begin{aligned} v_L = & \sum_{n=0}^{L-1} b_{2n+1} r^{n+1/2} \left\{ D \left( n + \frac{1}{2} \right) \left[ -\sin \left( n + \frac{1}{2} \right) \theta + \sin \left( n - \frac{3}{2} \right) \theta \right] \right\} \\ & + \sum_{n=1}^L r^n b_{2n} \{ (2-Dn)\sin n\theta + Dn\sin(n-2)\theta \}. \end{aligned} \quad (4.7)$$

Note that the number of unknown coefficients  $b_k$  in (4.6) and (4.7) is  $2L$  in  $u_L$  and  $v_L$ . Model A of crack singularity is said to be symmetric if  $u_y = v = 0$ , which implies that  $u$  in (4.6) and  $v$  in (4.7) are symmetric and anti-symmetric with respect to  $\theta = 0$ , respectively. We may also design the anti-symmetric Model A, by replacing the boundary conditions on  $\overline{OA}$  with

$$u = v_y = 0 \text{ on } \overline{OA}. \quad (4.8)$$

##### 4.2. The collocation Trefftz method

We will use the collocation method in [24] with the stability analysis [22].

We choose the particular solutions (4.6) and (4.7), and denote their set by  $V_L$ . Since they satisfy the governing equation (2.5) and the boundary conditions on  $\overline{OD} \cup \overline{OA}$  in Fig. 1 already, the coefficients  $a_k$  and  $b_k$  are sought by satisfying the rest of boundary conditions in (4.2)-(4.4). Define the boundary energy

$$I(u, v) = \int_{\overline{AB}} [(u-1)^2 + w^2 v_x^2] + \int_{\overline{BC}} [v^2 + w^2 u_y^2] + \int_{\overline{CD}} [u^2 + w^2 v_x^2], \quad (4.9)$$

where  $w$  is a weight. We choose  $w = 1/L$  in computation. The collocation Trefftz method (CTM) reads: To seek  $(u_N, v_N) \in V_L$  such that

$$I(u_L, v_L) = \min_{(u,v) \in V_L} I(u, v). \quad (4.10)$$

When the integrals in (4.9) involve numerical integration approximation, the solutions are given by

$$\hat{I}(u_L, v_L) = \min_{(u,v) \in V_L} \hat{I}(u, v), \quad (4.11)$$

<sup>2</sup> We use the notations  $u_y = \partial u / \partial y$  and  $v_x = \partial v / \partial x$ .

where

$$\hat{I}(u, v) = \int_{\overline{AB}} [(u-1)^2 + w^2 v_x^2] + \int_{\overline{BC}} [v^2 + w^2 u_y^2] + \int_{\overline{CD}} [u^2 + w^2 v_x^2], \tag{4.12}$$

where  $\hat{f}$  is the numerical approximation of  $f$  by some integration rules, such as the central or the Gaussian quadrature.

We may establish the collocation equations of  $u_L$  and  $v_L$ , to satisfy the rest of boundary conditions directly. Denote the basis particular solutions,

$$\begin{aligned} \Phi_{2n+1}(r, \theta) &= r^{n+1/2} [(2 + D(n + \frac{1}{2})) \cos(n + \frac{1}{2})\theta - D(n + \frac{1}{2}) \cos(n - \frac{3}{2})\theta], \\ \Phi_{2n}(r, \theta) &= r^n Dn [\cos n\theta - \cos(n-2)\theta], \end{aligned} \tag{4.13}$$

$$\begin{aligned} \Psi_{2n+1}(r, \theta) &= r^{n+1/2} D(n + \frac{1}{2}) [-\sin(n + \frac{1}{2})\theta + \sin(n - \frac{3}{2})\theta], \\ \Psi_{2n}(r, \theta) &= r^n [(2 - Dn) \sin n\theta + Dn \sin(n-2)\theta]. \end{aligned} \tag{4.14}$$

Then the solutions (4.6) and (4.7) are denoted simply by

$$u_L = \sum_{n=1}^{2L} b_n \Phi_n(r, \theta), \quad v_L = \sum_{n=1}^{2L} b_n \Psi_n(r, \theta). \tag{4.15}$$

Let  $\overline{AB} \cup \overline{BC} \cup \overline{CD}$  be divided uniformly into small sections with length  $h$ . Denote  $h = 1/N$ . On  $\overline{AB}$ , let  $Q_i = (r_i, \theta_i)$  denote the middle nodes of the small sections, we may have the following collocation equations from (4.4) and (4.15)

$$\sqrt{h} \sum_{n=1}^{2L} b_n \Phi_n(r_i, \theta_i) = \sqrt{h} \quad (r_i, \theta_i) \in \overline{AB}, \tag{4.16}$$

$$w\sqrt{h} \sum_{n=1}^{2L} b_n \frac{\partial}{\partial x} \Psi_n(r_i, \theta_i) = 0 \quad (r_i, \theta_i) \in \overline{AB}, \tag{4.17}$$

where  $w$  is a weight constant to balance two kinds of collocation equations. Similarly, from (4.2) and (4.3), we obtain the collocation equations on  $\overline{CB} \cup \overline{CD}$ ,

$$w\sqrt{h} \sum_{n=1}^{2L} b_n \frac{\partial}{\partial y} \Phi_n(r_i, \theta_i) = 0 \quad (r_i, \theta_i) \in \overline{BC}, \tag{4.18}$$

$$\sqrt{h} \sum_{n=1}^{2L} b_n \Psi_n(r_i, \theta_i) = 0 \quad (r_i, \theta_i) \in \overline{BC}, \tag{4.19}$$

$$\sqrt{h} \sum_{n=1}^{2L} b_n \Phi_n(r_i, \theta_i) = 0 \quad (r_i, \theta_i) \in \overline{CD}, \tag{4.20}$$

$$w\sqrt{h} \sum_{n=1}^{2L} b_n \frac{\partial}{\partial x} \Psi_n(r_i, \theta_i) = 0, \quad (r_i, \theta_i) \in \overline{CD}. \tag{4.21}$$

Eqs. (4.16)–(4.21) form the linear algebraic equations,

$$\mathbf{F}\mathbf{x} = \mathbf{b}, \tag{4.22}$$

where  $\mathbf{F} \in R^{m \times 2L}$ ,  $\mathbf{x} \in 2L$ ,  $\mathbf{b} \in R^m$ ,  $m = 8M$ , and  $M$  is the number of collocation nodes on  $\overline{AB}$ . We always choose  $m > 2L$  in computation. Hence Eq. (4.22) is the system of over-determined equations. We may choose the least squares method or the QR factorization, to seek the coefficient vector  $\mathbf{x} = \{b_1, b_2, \dots, b_{2L}\}^T$ . The CTM is studied in [24] for Poisson's, biharmonic and Helmholtz equations, as well as eigenvalue problems. This paper is the first one to apply the CTM for elastostatics problems. Following [24,31], the equivalence between (4.11) and (4.22) can be proven, and error bounds may also be derived. Details will appear elsewhere.

To measure numerical stability, we compute the condition number of (4.22) by

$$\text{Cond} = \frac{\sigma_{\max}}{\sigma_{\min}}, \tag{4.23}$$

where  $\sigma_{\max}$  and  $\sigma_{\min}$  are the maximal and the minimal singular values of matrix  $\mathbf{F}$ , respectively. A better criterion of numerical stability is given by the effective condition number in Li et al. [21], defined by

$$\text{Cond}_{\text{eff}} = \frac{\|\mathbf{b}\|}{\sigma_{\min} \|\mathbf{x}\|}, \tag{4.24}$$

where  $\|\mathbf{x}\|$  and  $\|\mathbf{b}\|$  are 2-norms.

### 4.3. Numerical results

We choose  $\lambda = \mu = 1$ , the constant  $D = \frac{1}{2}$  and  $\frac{5}{11}$  for the plane strain and the plane stress problems, respectively. Errors, condition numbers and the leading coefficients  $b_1$  and  $b_2$  are listed in Tables 1 and 2, where  $\delta(u) = u_L - u$  and  $\delta(v) = v_L - v$ . All tables of this paper are calculated by double precision. Table 3 lists the results of different  $M$  for  $D = \frac{1}{2}$  and  $L = 20$ , to find a good choice at  $M = 15$ . By using such a good matching:  $M = \frac{3}{4}L$ , the results are given in Tables 1 and 2. From Tables 1 and 2 we can see the asymptotes

$$\|\varepsilon\|_B = O(0.56^L), \quad \|\delta(u)\|_{\infty, \overline{AB} \cup \overline{CD}} = O(0.58^L), \quad \|\delta(v)\|_{\infty, \overline{BC}} = O(0.58^L), \tag{4.25}$$

$$\text{Cond} = O(1.48^L), \quad \text{Cond}_{\text{eff}} = O(1.06^L). \tag{4.26}$$

In Tables 4 and 5, we list the coefficients  $b_i$ , where the digits in bold denote the significant digits, compared with the higher accuracy of  $b_i$  obtained by using  $L = 200$  with 500 working digits. From Tables 4 and 5, we find that the crucial leading coefficient  $b_1$  has 14 significant digits.

## 5. Singular solutions near corners under free stress boundary conditions

For clamped boundary conditions the corner singularity of elasticity plane in Sections 3 and 4 are new. For free stress boundary conditions the corner singularity of elasticity plane was first discussed in Williams [38], and then in Lin and Tong [28], Jirousek and Wroblewski [15], Jirousek and Venkstesh [14] and Qin [36]. In this section, we consider only the symmetric case of the corner with  $\theta \in [0, \pm \Theta]$  for the sectorial domain  $S = \{(r, \theta) | 0 \leq R, 0 \leq \theta \leq \Theta\}$ , where  $\Theta \in (0, \pi]$ . The boundary conditions are given as follows:

$$u_y = v = 0 \quad \text{at } \theta = 0, \tag{5.1}$$

$$\tau_x = \tau_y = 0 \quad \text{at } \theta = \Theta, \tag{5.2}$$

where the stress formulas are given by

$$\tau_x = \sigma_x \cos(n, x) + \sigma_{xy} \cos(n, y), \tag{5.3}$$

$$\tau_y = \sigma_{xy} \cos(n, x) + \sigma_y \cos(n, y), \tag{5.4}$$

where  $\sigma_x = \sigma_{11}$ ,  $\sigma_y = \sigma_{22}$  and  $\sigma_{xy} = \sigma_{12} = \sigma_{21}$  as in (2.3). For the exterior normal  $\vec{n}$  of the edge boundary, we have

$$\cos(n, x) = \cos\left(\frac{\pi}{2} + \theta\right) = -\sin\theta, \quad \cos(n, y) = \cos\theta. \tag{5.5}$$

Hence we have from (5.3) and (5.4)

$$\tau_x = -\sigma_x \sin\theta + \sigma_{xy} \cos\theta = 0, \quad \tau_y = -\sigma_{xy} \sin\theta + \sigma_y \cos\theta = 0. \tag{5.6}$$

We will use different approaches from those in Sections 3 and 4, to seek the particular solutions of (3.2) and (3.3) satisfying the boundary conditions (5.1) and (5.6).

First, let us give a lemma from calculus.

**Table 1**

Errors, condition numbers and the leading coefficients  $b_1$  and  $b_2$  for Model A by the CTM as  $D = \frac{1}{2}$ .

$M$	$L$	$\ \varepsilon\ _B$	$\ \delta(u)\ _{\infty, \overline{AB \cup CD}}$	$\ \delta(v)\ _{\infty, \overline{BC}}$	Cond	Cond_eff	$b_1$	$b_2$
3	4	1.11(-2)	1.11(-2)	3.23(-3)	1.10(1)	1.72	4.10251982712500(-1)	2.50698260629164(-1)
6	8	6.79(-4)	7.35(-4)	3.21(-4)	8.23(1)	2.35	4.10654626510859(-1)	2.51537941297936(-1)
9	12	5.53(-5)	6.11(-5)	3.69(-5)	6.09(2)	3.54	4.10647631047238(-1)	2.51569676360375(-1)
12	16	4.96(-6)	5.50(-6)	3.57(-6)	3.93(3)	4.92	4.10647315855802(-1)	2.51571034567718(-1)
15	20	4.74(-7)	5.56(-7)	3.99(-7)	2.29(4)	6.38	4.10647274208211(-1)	2.51571057835755(-1)
18	24	4.68(-8)	6.12(-8)	4.55(-8)	1.24(5)	7.88	4.10647272484115(-1)	2.51571061809156(-1)
21	28	4.72(-9)	6.83(-9)	5.28(-9)	6.43(5)	9.41	4.10647272270177(-1)	2.51571061743769(-1)
24	32	4.84(-10)	7.60(-10)	6.13(-10)	3.21(6)	11	4.10647272267430(-1)	2.51571061764127(-1)
27	36	5.01(-11)	8.59(-11)	7.11(-11)	1.56(7)	12.5	4.10647272266421(-1)	2.51571061762613(-1)
30	40	5.25(-12)	9.84(-12)	8.23(-12)	7.41(7)	14.1	4.10647272266494(-1)	2.51571061762734(-1)

**Table 2**

Errors, condition numbers and the leading coefficients  $b_1$  and  $b_2$  for Model A by the CTM as  $D = \frac{5}{11}$ .

$M$	$L$	$\ \varepsilon\ _B$	$\ \delta(u)\ _{\infty, \overline{AB \cup CD}}$	$\ \delta(v)\ _{\infty, \overline{BC}}$	Cond	Cond_eff	$b_1$	$b_2$
3	4	1.11(-2)	1.19(-2)	2.92(-3)	9.27	1.63	4.07637079571382(-1)	2.08971959209034(-1)
6	8	6.33(-4)	6.98(-4)	2.88(-4)	7.02(1)	2.3	4.07798355978068(-1)	2.09715764414755(-1)
9	12	5.10(-5)	5.76(-5)	3.39(-5)	5.16(2)	3.46	4.07792974425139(-1)	2.09740315069872(-1)
12	16	4.56(-6)	5.17(-6)	3.27(-6)	3.32(3)	4.79	4.07792708870668(-1)	2.09741269589606(-1)
15	20	4.35(-7)	5.16(-7)	3.62(-7)	1.93(4)	6.21	4.07792671891129(-1)	2.09741282540668(-1)
18	24	4.29(-8)	5.51(-8)	4.11(-8)	1.05(5)	7.68	4.07792670357375(-1)	2.09741285227151(-1)
21	28	4.33(-9)	6.21(-9)	4.77(-9)	5.44(5)	9.17	4.07792670157548(-1)	2.09741285168149(-1)
24	32	4.43(-10)	6.95(-10)	5.55(-10)	2.72(6)	10.7	4.07792670153856(-1)	2.09741285183747(-1)
27	36	4.59(-11)	7.78(-11)	6.46(-11)	1.32(7)	12.2	4.07792670152782(-1)	2.09741285182719(-1)
30	40	4.80(-12)	8.93(-12)	7.50(-12)	6.28(7)	13.7	4.07792670152828(-1)	2.09741285182831(-1)

**Table 3**

Errors, condition numbers and the leading coefficients  $b_1$  and  $b_2$  for model A by the CTM as  $D = \frac{1}{2}$  and  $L = 20$ .

$M$	$\ \varepsilon\ _B$	$\ \delta(u)\ _{\infty, \overline{AB \cup CD}}$	$\ \delta(v)\ _{\infty, \overline{BC}}$	Cond	Cond_eff	$b_1$	$b_2$
6	2.57(-7)	4.67(-6)	2.55(-6)	2.21(4)	6.58	4.10647249074947(-1)	2.51571038577816(-1)
9	4.30(-7)	1.25(-6)	8.18(-7)	2.26(4)	6.41	4.10647271576030(-1)	2.51571053945849(-1)
12	4.63(-7)	7.22(-7)	5.12(-7)	2.28(4)	6.39	4.10647273644057(-1)	2.51571056781410(-1)
15	4.74(-7)	5.56(-7)	3.99(-7)	2.29(4)	6.38	4.10647274208211(-1)	2.51571057835755(-1)
18	4.80(-7)	5.28(-7)	3.51(-7)	2.29(4)	6.38	4.10647274441860(-1)	2.51571058364345(-1)
21	4.83(-7)	5.14(-7)	3.54(-7)	2.30(4)	6.37	4.10647274561803(-1)	2.51571058671756(-1)
24	4.85(-7)	5.06(-7)	3.55(-7)	2.30(4)	6.37	4.10647274631963(-1)	2.51571058867490(-1)
27	4.86(-7)	5.01(-7)	3.56(-7)	2.30(4)	6.37	4.10647274676761(-1)	2.51571059000162(-1)
30	4.87(-7)	4.97(-7)	3.57(-7)	2.30(4)	6.37	4.10647274707216(-1)	2.51571059094364(-1)
33	4.88(-7)	4.94(-7)	3.57(-7)	2.30(4)	6.37	4.10647274728920(-1)	2.51571059163711(-1)

**Table 4**

The leading coefficients for Model A by the CTM as  $D = \frac{1}{2}$  and  $L = 40$ , where "Sig." denotes the number of significant digits.

$i$	All digits	Sig.	$i$	All digits	Sig.
1	<b>4.10647272266494</b> (-1)	14	41	<b>6.66203785143277</b> (-10)	2
2	<b>2.51571061762734</b> (-1)	13	42	<b>1.15945989171034</b> (-8)	5
3	<b>9.38402559696613</b> (-2)	15	43	<b>-9.60009004400285</b> (-9)	3
4	<b>3.07049108695358</b> (-2)	12	44	<b>6.04311295359092</b> (-9)	3
5	<b>-1.33072821410658</b> (-2)	13	45	<b>-1.76376133662528</b> (-10)	3
6	<b>-2.60551082272462</b> (-3)	11	46	<b>-2.78038068295960</b> (-9)	3
7	<b>8.91754903117311</b> (-3)	11	47	<b>2.83403346078824</b> (-9)	2
8	<b>-6.69374333311149</b> (-3)	11	48	<b>-1.44186153642238</b> (-9)	3
9	<b>3.64135235015083</b> (-6)	7	49	<b>3.14476054802687</b> (-11)	1
10	<b>1.32797603164819</b> (-3)	10	50	<b>6.67848883183302</b> (-10)	2
11	<b>-3.93851124649121</b> (-4)	10	51	<b>-5.70188229184457</b> (-10)	2
12	<b>8.37925322844148</b> (-4)	10	52	<b>3.44387135360899</b> (-10)	1
13	<b>-2.99456416826879</b> (-6)	8	53	<b>-8.58589386862836</b> (-12)	1
14	<b>-3.52429864903714</b> (-4)	10	54	<b>-1.59990818166583</b> (-10)	2
15	<b>3.24690762531300</b> (-4)	9	55	<b>1.62417585401824</b> (-10)	2
16	<b>-1.57456292034103</b> (-4)	9	56	<b>-8.21171983972912</b> (-11)	1

Table 4 (continued)

<i>i</i>	All digits	Sig.	<i>i</i>	All digits	Sig.
17	-8.06323240445713(-8)	8	57	1.32661996826598(-12)	0
18	7.13124976192084(-5)	8	58	3.84098467576664(-11)	1
19	-4.08204603518075(-5)	8	59	-3.30585176206119(-11)	1
20	3.86936544039162(-5)	9	60	1.90642054386328(-11)	1
21	-3.37456775389799(-6)	8	61	-2.76785227660824(-13)	0
22	-1.53715418109543(-5)	9	62	-8.85386594129221(-12)	1
23	1.65849368076219(-5)	8	63	8.82299324111171(-12)	0
24	-8.79407462804704(-6)	7	64	-4.35547407571365(-12)	0
25	3.55797930227532(-7)	7	65	1.09378835982087(-14)	0
26	3.66941729535458(-6)	7	66	2.03763392558782(-12)	1
27	-2.65431844360349(-6)	6	67	-1.68106310329655(-12)	1
28	1.96251164309636(-6)	7	68	8.69138399475569(-13)	0
29	-7.44579741160189(-8)	5	69	2.96650892492666(-14)	0
30	-8.73166822322096(-7)	6	70	-3.98231056553442(-13)	0
31	8.91881164790093(-7)	5	71	3.59990602781704(-13)	0
32	-4.55641167759193(-7)	5	72	-1.61383266794306(-13)	0
33	1.19919047833575(-8)	5	73	-2.52343313378514(-15)	0
34	2.04817784788004(-7)	6	74	7.17469564382971(-14)	0
35	-1.60762427325796(-7)	5	75	-5.29166570542841(-14)	0
36	1.07730530862914(-7)	6	76	2.03478061859853(-14)	0
37	-4.04907284800504(-9)	5	77	4.94681464457029(-15)	0
38	-4.84832287196653(-8)	4	78	-9.46006769817745(-15)	0
39	4.97724966182189(-8)	4	79	5.00214195594415(-15)	0
40	-2.54740240872724(-8)	3	80	-8.07198239900287(-16)	0

Table 5

The leading coefficients for Model A by the CTM as  $D = \frac{\pi}{11}$  and  $L = 40$ , where "Sig." denotes the number of significant digits.

<i>i</i>	All digits	Sig.	<i>i</i>	All digits	Sig.
1	4.07792670152828(-1)	14	41	6.05177310086391(-10)	2
2	2.09741285182831(-1)	13	42	1.04536619410337(-8)	4
3	9.62856103741701(-2)	13	43	-8.46189297869868(-9)	3
4	2.76183879928471(-2)	12	44	5.46078789055957(-9)	3
5	-1.19762989485270(-2)	13	45	-1.64827249646381(-10)	4
6	-2.33509762908167(-3)	11	46	-2.50707482281544(-9)	3
7	8.01687784228362(-3)	11	47	2.55828004544452(-9)	3
8	-6.02281434461713(-3)	11	48	-1.30289694456698(-9)	3
9	-1.49130657247683(-4)	9	49	2.88341445649076(-11)	2
10	1.19033908130327(-3)	11	50	6.02263711566840(-10)	2
11	-2.33838564513919(-4)	10	51	-5.04926183157911(-10)	1
12	7.55317190747609(-4)	10	52	3.11217597658468(-10)	2
13	-4.67818188128383(-6)	8	53	-8.07135498822367(-12)	1
14	-3.16076477511216(-4)	10	54	-1.44261350147204(-10)	1
15	2.92274339687023(-4)	10	55	1.46628619946623(-10)	2
16	-1.42308887670281(-4)	10	56	-7.42282143061901(-11)	0
17	-9.27204750148185(-8)	6	57	1.25995634010131(-12)	0
18	6.40450310193123(-5)	8	58	3.46340109145301(-11)	0
19	-3.38908825256940(-5)	9	59	-2.93769250151829(-11)	1
20	3.49934776471675(-5)	9	60	1.72565018449597(-11)	1
21	-3.13138520671870(-6)	7	61	-2.89465586997051(-13)	0
22	-1.38289869832243(-5)	8	62	-7.97136045559816(-12)	0
23	1.49653027384342(-5)	8	63	7.96654251154383(-12)	0
24	-7.95240917059638(-6)	7	64	-3.94305407358385(-12)	1
25	3.12150940793412(-7)	7	65	2.04069440948681(-14)	0
26	3.30450798386428(-6)	7	66	1.83503653391467(-12)	1
27	-2.28920507763688(-6)	7	67	-1.49968276847398(-12)	0
28	1.77429904226567(-6)	7	68	7.93632735452351(-13)	0
29	-7.01146877282463(-8)	6	69	1.95426543505334(-14)	0
30	-7.86691120596366(-7)	6	70	-3.55692004814227(-13)	0
31	8.04971808535152(-7)	6	71	3.24815277110125(-13)	0
32	-4.11865176466270(-7)	5	72	-1.46421721827824(-13)	0
33	1.08394196959857(-8)	4	73	-1.40547698795339(-15)	0
34	1.84587751771489(-7)	6	74	6.44070271052293(-14)	0
35	-1.40643173854669(-7)	5	75	-4.74784256728221(-14)	0
36	9.73672023784266(-8)	4	76	1.91906722780754(-14)	0
37	-3.77658521611431(-9)	4	77	3.80036316303079(-15)	0
38	-4.37046290153343(-8)	5	78	-8.19514263828618(-15)	0
39	4.49271969353005(-8)	4	79	4.46795073904157(-15)	0
40	-2.30212113988927(-8)	4	80	-7.19945688720858(-16)	0



**Lemma 5.1.** *There exist the equalities,*

$$\frac{\partial r}{\partial x} = \cos\theta, \quad \frac{\partial r}{\partial y} = \sin\theta,$$

$$\frac{\partial \theta}{\partial x} = -\frac{\sin\theta}{r}, \quad \frac{\partial \theta}{\partial y} = \frac{\cos\theta}{r}. \quad (5.7)$$

**Lemma 5.2.** *For the solutions (3.2) and (3.3) satisfying (5.1), the stress tensors  $\tau_x$  and  $\tau_y$  at  $\theta \in (0, \theta]$  are given by*

$$\tau_x = 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [Dv_k \sin(v_k-2)\theta - D \sin v_k \theta] + d_k \sin v_k \theta\}, \quad (5.8)$$

$$\tau_y = 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [Dv_k \cos(v_k-2)\theta + D \cos v_k \theta] + d_k \cos v_k \theta\}. \quad (5.9)$$

**Proof.** Since

$$u_y = \frac{\partial u}{\partial y} = \frac{\partial u}{\partial r} \frac{\partial r}{\partial y} + \frac{\partial u}{\partial \theta} \frac{\partial \theta}{\partial y}, \quad (5.10)$$

we have from (3.2) and Lemma 5.1

$$u_y = \sum_{k=1}^L v_k r^{v_k-1} \{a_k [-\cos(v_k-1)\theta + Dv_k \cos(v_k-3)\theta - 2D \cos(v_k-2)\theta \cos\theta] + b_k [-\sin(v_k-1)\theta + Dv_k \sin(v_k-3)\theta - 2D \sin(v_k-2)\theta \cos\theta] + c_k \cos(v_k-1)\theta + d_k \sin(v_k-1)\theta\}. \quad (5.11)$$

When  $u_y = \partial u / r \partial \theta|_{\theta=0} = 0$  we obtain

$$\sum_{k=1}^L v_k r^{v_k-1} \{a_k [-1 + Dv_k - 2D] + c_k\} = 0, \quad (5.12)$$

to give

$$c_k = -(-1 + Dv_k - 2D)a_k, \quad k = 1, 2, \dots \quad (5.13)$$

On the other hand, when the boundary condition  $v = 0|_{\theta=0}$ , we have from (3.3)

$$c_0 = 0, \quad c_k = -(1 + Dv_k)a_k, \quad k = 1, 2, \dots \quad (5.14)$$

Eqs. (5.13) and (5.14) lead to

$$c_k = a_k = 0, \quad \forall k. \quad (5.15)$$

From (5.15), the particular solutions in (3.2) and (3.3) are simplified to

$$u_L = \sum_{k=1}^L r^{v_k} \{b_k [\cos v_k \theta - Dv_k \cos(v_k-2)\theta] - d_k \cos v_k \theta\} + d_0, \quad (5.16)$$

$$v_L = \sum_{k=1}^L r^{v_k} \{b_k [\sin v_k \theta + Dv_k \sin(v_k-2)\theta] + d_k \sin v_k \theta\}. \quad (5.17)$$

Next, based on Lemma 5.1, we have from (5.10) and (5.16)

$$u_y = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [-\sin(v_k-1)\theta + Dv_k \sin(v_k-3)\theta - 2D \sin(v_k-2)\theta \cos\theta] + d_k \sin(v_k-1)\theta\} \\ = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [-(1+D)\sin(v_k-1)\theta + D(v_k-1)\sin(v_k-3)\theta] + d_k \sin(v_k-1)\theta\}, \quad (5.18)$$

where we have used  $2\sin(v_k-2)\theta \cos\theta = \sin(v_k-1)\theta + \sin(v_k-3)\theta$ . Similarly, we have

$$v_x = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [\sin(v_k-1)\theta + Dv_k \sin(v_k-3)\theta + 2D \cos(v_k-2)\theta \sin\theta] + d_k \sin(v_k-1)\theta\}$$

$$= \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(1+D)\sin(v_k-1)\theta + D(v_k-1)\sin(v_k-3)\theta] + d_k \sin(v_k-1)\theta\}. \quad (5.19)$$

Then the stress is given by

$$\sigma_{xy} = \sigma_{12} \\ = \mu(u_y + v_x) \\ = 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [D(v_k-1)\sin(v_k-3)\theta] + d_k \sin(v_k-1)\theta\}. \quad (5.20)$$

Third, based on Lemma 5.1, we have from (5.16) and (5.17)

$$u_x = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [\cos(v_k-1)\theta - Dv_k \cos(v_k-3)\theta + 2D \sin(v_k-2)\theta \sin\theta] - d_k \cos(v_k-1)\theta\} \\ = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(1-D)\cos(v_k-1)\theta - D(v_k-1)\cos(v_k-3)\theta] - d_k \cos(v_k-1)\theta\}, \quad (5.21)$$

$$v_y = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [\cos(v_k-1)\theta + Dv_k \cos(v_k-3)\theta - 2D \cos(v_k-2)\theta \cos\theta] + d_k \cos(v_k-1)\theta\} \\ = \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(1-D)\cos(v_k-1)\theta + D(v_k-1)\cos(v_k-3)\theta] + d_k \cos(v_k-1)\theta\}. \quad (5.22)$$

Then we have (5.21) and (5.22)

$$\sigma_x = \sigma_{11} \\ = (\lambda + 2\mu)u_x + \lambda v_y \\ = 2 \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(\lambda + \mu)(1-D)\cos(v_k-1)\theta - \mu D(v_k-1)\cos(v_k-3)\theta] - d_k [\mu \cos(v_k-1)\theta]\}, \quad (5.23)$$

and

$$\sigma_y = \sigma_{22} = \lambda u_x + (\lambda + 2\mu)v_y \\ = 2 \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(\lambda + \mu)(1-D)\cos(v_k-1)\theta + \mu D(v_k-1)\cos(v_k-3)\theta] + d_k [\mu \cos(v_k-1)\theta]\}. \quad (5.24)$$

Therefore, we have (5.20) and (5.23)

$$\tau_x = -\sigma_x \sin\theta + \sigma_{xy} \cos\theta \\ = 2 \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(-\lambda(1-D) - \mu(1-D))\cos(v_k-1)\theta \sin\theta + \mu D(v_k-1)\sin(v_k-2)\theta] + d_k \mu \sin v_k \theta\}. \quad (5.25)$$

From  $D = (\lambda + \mu)/(\lambda + 3\mu)$ , there exists the equality

$$\lambda(1-D) = \mu(3D-1). \quad (5.26)$$

Eq. (5.25) is reduced to

$$\tau_x = 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [-2D \cos(v_k-1)\theta \sin\theta + D(v_k-1)\sin(v_k-2)\theta] + d_k \sin v_k \theta\}$$

$$= 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [Dv_k \sin(v_k-2)\theta - D \sin v_k \theta] + d_k \sin v_k \theta\}. \tag{5.27}$$

This is the first desired result (5.8).

From (5.20), (5.24) and (5.26), we have similarly

$$\begin{aligned} \tau_y &= -\sigma_{xy} \sin \theta + \sigma_y \cos \theta \\ &= 2 \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(\lambda(1-D) + \mu(1-D)) \cos(v_k-1)\theta \cos \theta \\ &\quad + \mu D(v_k-1) \cos(v_k-2)\theta] + d_k \mu \cos v_k \theta\} \\ &= 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [D \cos(v_k-1)\theta \cos \theta \\ &\quad + 2D(v_k-1) \cos(v_k-2)\theta] + d_k \cos v_k \theta\} \\ &= 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [Dv_k \cos(v_k-2)\theta + D \cos v_k \theta] + d_k \cos v_k \theta\}. \end{aligned} \tag{5.28}$$

This is the second desired result (5.9), and completes the proof of Lemma 5.2. □

**Theorem 5.1.** For (5.1) and (5.2) with symmetric cases there exists the equality,

$$\sin 2v^* \Theta = -v^* \sin 2\Theta. \tag{5.29}$$

**Proof.** Based on Lemma 5.2, the nonzero coefficients  $b_k$  and  $d_k$  in  $\tau_x = \tau_y = 0$  exist if and only if the coefficient determinant in (5.8) and (5.9) is zero, to give

$$[Dv_k \sin(v_k-2)\theta - D \sin v_k \theta] \times \cos v_k \theta - [Dv_k \cos(v_k-2)\theta + D \cos v_k \theta] \times \sin v_k \theta = 0. \tag{5.30}$$

This yields

$$v_k [\sin(v_k-2)\theta \cos v_k \theta - \cos(v_k-2)\theta \sin v_k \theta] - \sin 2v_k \theta = -v_k \sin 2\theta - \sin 2v_k \theta = 0. \tag{5.31}$$

This is the desired result (5.29), and completes the proof of Theorem 5.1. □

In fact, Eq. (5.29) corresponds to the symmetric model with (5.16) and (5.17), and the following equation:

$$\sin 2v^* \Theta = v^* \sin 2\Theta \tag{5.32}$$

corresponds to the nonsymmetric model with  $v_y = u = 0$  at  $\theta = 0$ . The particular solutions can also be obtained similarly, and the proof techniques are similar. Eqs. (5.29) and (5.32) coincide with [28]. Note that the signs corresponding the symmetry and anti-symmetry in Theorem 5.1 for the free stress boundary at  $\theta = \Theta$  are just opposite to those in Theorem 3.1 for the clamped boundary conditions on  $\theta = \Theta$ .

Compared with Theorem 3.1, since  $D \in (0, 1)$ , it is easy to see that the minimal real root of (3.13) are larger than that of (5.29) and (5.32). This coincides with the fact that the clamped boundary condition is a kind of constraints, so that the minimal eigenvalue from (3.13) will be larger.

**Remark 5.1.** Although Lemma 5.2 and Theorem 5.1 are derived for the plane strain problem, they are still valid for the plane stress problems, since for the latter case, only the Lamé constants  $\lambda$  is replaced by  $\lambda' = 2\lambda\mu/(\lambda+2\mu)$  (see [40]).

**Remark 5.2.** Note that in Section 3 and this section, the analytical approaches for singular particular solutions are *different*. Based on the general particular solutions (3.2) and (3.3) satisfying the Cauchy-Navier equations (2.14) and (2.15), in Section 3, for the

corners with  $|\theta| \leq \Theta \leq \pi$ , first we obtain the particular solutions satisfying the boundary conditions at  $\theta = \pm \Theta$ , and then select the final particular solutions satisfying the boundary conditions at  $\theta = 0$ . In this section, the orders to satisfy the boundary conditions are switched. The boundary conditions at  $\theta = 0$  are first required, and those at  $\theta = \Theta$  are then added into. In both approaches, the separation techniques in dealing with the complicated boundary conditions are developed to greatly simplify the derivation for singular properties and particular solutions at corners of linear elastostatics, which are derived by calculus. Hence, one of the contributions of this paper is such an effective systematic analysis, which enriches both the theory of linear elastostatics and its numerical methods.

**6. Model B of crack singularity and its numerical solutions**

*6.1. Model B of crack singularity*

In this section, let us consider the interior crack of symmetric models with free stress for (5.1) and (5.6) at  $\Theta = \pi$ :

$$v = u_y = 0 \quad \text{at } \theta = 0, \tag{6.1}$$

$$\sigma_{xy} = 0, \quad \sigma_y = 0 \quad \text{at } \theta = \pi. \tag{6.2}$$

From the boundary conditions (6.2) at  $\theta = \pi$  we have

$$\sigma_{xy} = 2\mu \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(D(v_k-1)) \sin(v_k-3)\pi] + d_k \sin(v_k-1)\pi\} = 0, \tag{6.3}$$

$$\begin{aligned} \sigma_y &= 2 \sum_{k=1}^L v_k r^{v_k-1} \{b_k [(\lambda + \mu)(1-D) \cos(v_k-1)\pi \\ &\quad + \mu D(v_k-1) \cos(v_k-3)\pi] + d_k [\mu \cos(v_k-1)\pi]\} = 0. \end{aligned} \tag{6.4}$$

Based on Theorem 5.1, when  $\Theta = \pi$ , we also conclude that  $v^* = n$  and  $v^* = n + \frac{1}{2}$ . The particular solutions are split into two cases: (1)  $v^* = n + \frac{1}{2}$  and (2)  $v^* = n$ . First for  $v^* = n + \frac{1}{2}$ , from (6.3) we have the relations of coefficients

$$d_{2n+1} = -D(n-\frac{1}{2})b_{2n+1}, \tag{6.5}$$

and for  $v^* = n$  from (6.4),

$$d_{2n} = \frac{-(\lambda + \mu)(1-D) + \mu D(1-n)}{\mu} b_{2n}. \tag{6.6}$$

Then the solutions (5.16) and (5.17) lead to

$$\begin{aligned} u_L &= \sum_{n=0}^{L-1} r^{n+1/2} b_{2n+1} \left[ \left( D \left( n - \frac{1}{2} \right) + 1 \right) \cos \left( n + \frac{1}{2} \right) \theta \right. \\ &\quad \left. - D \left( n + \frac{1}{2} \right) \cos \left( n - \frac{3}{2} \right) \theta \right] \\ &\quad + \sum_{n=1}^L r^n b_{2n} \left[ \frac{\lambda(1-D) + 2\mu(1-D) + \mu D n}{\mu} \cos n \theta - D n \cos(n-2)\theta \right] \\ &\quad + b_0, \end{aligned} \tag{6.7}$$

$$\begin{aligned} v_L &= \sum_{n=0}^{L-1} r^{n+1/2} b_{2n+1} \left[ \left( D \left( -n + \frac{1}{2} \right) + 1 \right) \sin \left( n + \frac{1}{2} \right) \theta \right. \\ &\quad \left. + D \left( n + \frac{1}{2} \right) \sin \left( n - \frac{3}{2} \right) \theta \right] \\ &\quad + \sum_{n=1}^L r^n b_{2n} \left[ \frac{-\lambda(1-D) + 2\mu D - \mu D n}{\mu} \sin n \theta + D n \sin(n-2)\theta \right]. \end{aligned} \tag{6.8}$$

By means of (5.26), the final particular solutions are obtained:

$$u_L = \sum_{n=0}^{L-1} r^{n+1/2} \left\{ b_{2n+1} \left[ \left( D \left( n - \frac{1}{2} \right) + 1 \right) \cos \left( n + \frac{1}{2} \right) \theta - D \left( n + \frac{1}{2} \right) \cos \left( n - \frac{3}{2} \right) \theta \right] + \sum_{n=1}^L r^n \{ b_{2n} [(D(n+1)+1)\cos n\theta - Dn\cos(n-2)\theta] \} + b_0, \right. \quad (6.9)$$

$$v_L = \sum_{n=0}^{L-1} r^{n+1/2} \left\{ b_{2n+1} \left[ \left( D \left( -n + \frac{1}{2} \right) + 1 \right) \sin \left( n + \frac{1}{2} \right) \theta + D \left( n + \frac{1}{2} \right) \sin \left( n - \frac{3}{2} \right) \theta \right] + \sum_{n=1}^L r^n \{ b_{2n} [(-D(n+1)+1)\sin n\theta + Dn\sin(n-2)\theta] \}. \right. \quad (6.10)$$

6.2. Numerical results

When the particular solutions (6.9) and (6.10) replace (4.6) and (4.7) in Model A, we obtain Model B with the following boundary conditions:

$$\sigma_{xy} = \sigma_y = 0 \quad \text{on } \overline{OD}, \quad (6.11)$$

$$v = u_y = 0 \quad \text{on } \overline{OA} \cup \overline{CB}, \quad (6.12)$$

$$u = v_x = 0 \quad \text{on } \overline{CD}, \quad (6.13)$$

$$u = 1, \quad v_x = 0 \quad \text{on } \overline{AB}. \quad (6.14)$$

Since the particular solutions (6.9) and (6.10) satisfy the Cauchy-Navier equation (2.5) and the boundary conditions on  $\overline{OA} \cup \overline{OD}$  already, the coefficients  $b_i$  are sought by satisfying the rest boundary conditions in (6.12)–(6.14). The same CTM in Section 4.2 is also valid for Model B. The errors and condition numbers are

listed in Tables 6–9, from which we can find the same rates as in (4.25) and (4.26).

7. Leading powers  $v_k$  of the corner solutions  $O(r^{v_k})$

7.1. Algorithms

In this section, we study the leading  $v_k = \alpha_k + i\beta_k$  for the corner solutions, where  $\alpha_k, \beta_k \in \mathfrak{R}$ . Consider (3.13) as an example, since Eq. (5.29) is a special case at  $D = 1$  with sign “–”. For complex roots of  $v^* = \alpha + i\beta$ , we convert (3.13) to two real equations of  $\alpha$  and  $\beta$  as follows.

Since

$$\sin z = \frac{e^{iz} - e^{-iz}}{2i}, \quad (7.1)$$

we obtain from (3.13)

$$\frac{e^{i2(\alpha+i\beta)\theta} - e^{-i2(\alpha+i\beta)\theta}}{2i} = \pm D(\alpha+i\beta)\sin 2\theta. \quad (7.2)$$

For the real part of (7.2) we have

$$\frac{e^{-2\beta\theta} + e^{2\beta\theta}}{2} \sin 2\alpha\theta = \pm D\alpha \sin 2\theta, \quad (7.3)$$

to give

$$\cosh(2\beta\theta)\sin 2\alpha\theta = \pm D\alpha \sin 2\theta. \quad (7.4)$$

For the imaginary part of (7.2), we have

$$\frac{e^{2\beta\theta} - e^{-2\beta\theta}}{2} \cos 2\alpha\theta = \pm D\beta \sin 2\theta,$$

to give

$$\sinh(2\beta\theta)\cos 2\alpha\theta = \pm D\beta \sin 2\theta. \quad (7.5)$$

Then we may apply the Newton iteration, to seek two real  $\alpha$  and  $\beta$  by satisfying (7.4) and (7.5).

Table 6

Errors, condition numbers and the leading coefficients  $b_1$  and  $b_0$  for Model B by the CTM as  $D = \frac{1}{2}$ .

M	L	$\  \varepsilon \ _B$	$\  \delta(u) \ _{\infty, \overline{AB} \cup \overline{CD}}$	$\  \delta(v) \ _{\infty, \overline{BC}}$	Cond	Cond_eff	$b_1$	$b_0$
3	4	6.42(−3)	5.71(−3)	4.92(−3)	1.88(1)	2.37	0.190614727092869	0.460409333521642
6	8	2.60(−4)	2.19(−4)	2.36(−4)	1.68(2)	3.92	0.191436302772700	0.460488642952438
9	12	2.67(−5)	2.77(−5)	2.40(−5)	1.24(3)	5.95	0.191431830741538	0.460491701207298
12	16	2.51(−6)	2.71(−6)	2.23(−6)	7.92(3)	8.23	0.191430875888065	0.460492795062574
15	20	2.58(−7)	2.96(−7)	2.37(−7)	4.61(4)	10.7	0.191430793209968	0.460492860222083
18	24	2.69(−8)	3.29(−8)	2.64(−8)	2.50(5)	13.2	0.191430788743211	0.460492862324388
21	28	2.85(−9)	3.69(−9)	3.01(−9)	1.30(6)	15.8	0.191430788341483	0.460492862332309
24	32	3.02(−10)	4.16(−10)	3.44(−10)	6.47(6)	18.5	0.191430788360548	0.460492862295949
27	36	3.21(−11)	4.70(−11)	3.92(−11)	3.14(7)	21.1	0.191430788363848	0.460492862291750
30	40	3.43(−12)	5.34(−12)	4.46(−12)	1.49(8)	23.8	0.191430788364732	0.460492862291075

Table 7

Errors, condition numbers and the leading coefficients  $b_1$  and  $b_0$  for Model B by the CTM as  $D = \frac{5}{11}$ .

M	L	$\  \varepsilon \ _B$	$\  \delta(u) \ _{\infty, \overline{AB} \cup \overline{CD}}$	$\  \delta(v) \ _{\infty, \overline{BC}}$	Cond	Cond_eff	$b_1$	$b_0$
3	4	4.57(−3)	4.07(−3)	3.66(−3)	1.78(1)	2.46	0.142950891214175	0.464934155574646
6	8	1.82(−4)	1.50(−4)	1.67(−4)	1.57(2)	4.06	0.143640461098265	0.464855368553578
9	12	1.86(−5)	1.93(−5)	1.72(−5)	1.16(3)	6.15	0.143639690785542	0.464854952373997
12	16	1.74(−6)	1.89(−6)	1.60(−6)	7.36(3)	8.47	0.143638839420185	0.464855849789067
15	20	1.78(−7)	2.08(−7)	1.70(−7)	4.27(4)	11.0	0.143638760656728	0.464855919706794
18	24	1.86(−8)	2.31(−8)	1.89(−8)	2.33(5)	13.6	0.143638755600059	0.464855923599455
21	28	1.96(−9)	2.60(−9)	2.16(−9)	1.20(6)	16.3	0.143638755231839	0.464855923749553
24	32	2.08(−10)	2.94(−10)	2.47(−10)	6.02(6)	19.0	0.143638755246206	0.464855923724844
27	36	2.21(−11)	3.33(−11)	2.82(−11)	2.92(7)	21.8	0.143638755249903	0.464855923720447
30	40	2.36(−12)	3.79(−12)	3.22(−12)	1.39(8)	24.6	0.143638755250735	0.464855923719710

7.2. Numerical results for  $\Theta = \pi/4, 3\pi/4$

The constants  $D = \frac{1}{2}$  and  $\frac{5}{11}$  correspond to the corners under the clamped boundary conditions, and  $D = 1$  to those under free stress boundary conditions. We only investigate two cases numerically: (1)  $\Theta = \pi/4$  and (2)  $\Theta = 3\pi/4$ , which correspond to the rectangular corners and the concave corners of L-shaped domains, respectively. Note that such two cases are often used in engineering applications.

First  $\Theta = \pi/4$ , the leading  $v_k$  are listed in Tables 10–12 for  $D = \frac{1}{2}, \frac{5}{11}, 1$ , respectively, where “symmetric” and “anti-symmetric” denote the symmetric and the anti-symmetric models of crack singularity, respectively. The minimal  $v_1$  is real, to have

$$v_1 = \alpha_1 = 1.473 \quad \text{for } D = \frac{1}{2}, \tag{7.6}$$

$$v_1 = \alpha_1 = 1.516 \quad \text{for } D = \frac{5}{11}, \tag{7.7}$$

$$v_1 = \alpha_1 = 1 \quad \text{for } D = 1. \tag{7.8}$$

From (7.6) and (7.7), although the derivatives of solutions  $u$  and  $v$  at the rectangular angles are bounded for the clamped boundary conditions, the second order derivatives are unbounded. Since for  $u = O(r^p)$ , where  $p (> 1)$  is noninteger,  $u \in H^{p+1-\delta}(S)$ ,  $0 < \delta \ll 1$ . We conclude that  $u, v \in H^2(S)$  but not in  $H^3(S)$ . This implies that for the corners with clamped boundary conditions, the linear elements

may retain the optimal convergence rates  $O(h)$  in  $H^1$  norm (see [2]), where  $h$  is the maximal boundary length of quasi-uniform triangular elements. However, high order elements will lead to reduced convergence rates. For the rectangular corners with free stress boundary conditions, since  $v_2 = 2.739 \pm 1.119i$  with the solution

$$u = O(|r^{v_2}|) = O(r^{2.2}) = O(r^{2.739}), \tag{7.9}$$

the solutions  $u, v \in H^3(S)$  but not in  $H^4(S)$ . The quadratic elements retain the optimal convergence rates  $O(h^2)$  in  $H^1$  norm, but the cubic elements will lead to reduction of convergence rates.

For the L-shaped domains, the leading  $v_k$  are listed in Tables 13–15. We can see that the minimal  $v_1$  are also real, with

$$v_1 = \alpha_1 = 0.6018 \quad \text{for } D = \frac{1}{2}, \tag{7.10}$$

$$v_1 = \alpha_1 = 0.6073 \quad \text{for } D = \frac{5}{11}, \tag{7.11}$$

$$v_1 = \alpha_1 = 0.5445 \quad \text{for } D = 1. \tag{7.12}$$

For  $\Theta = 3\pi/4$  and  $\Theta = \pi$ , all elements including linear elements suffer from reduction of convergence rates. Special treatments, such as local refinements or some combining techniques, must be done to recover the optimal convergence rates. Note that the leading  $v_k$  in Table 15 for symmetric Model B are consistent with those in [28].

**Table 8**  
The leading coefficients for Model B by the CTM as  $D = \frac{1}{2}$  and  $L = 40$ , where “Sig.” denotes the number of significant digits.

$i$	All digits	Sig.	$i$	All digits	Sig.
0	<b>4.60492862291</b> 075(−1)	12	41	− <b>1.73245721041029</b> (−9)	3
1	<b>1.91430788364</b> 732(−1)	12	42	− <b>1.03956126266378</b> (−8)	5
2	<b>2.85280692235</b> 259(−1)	11	43	<b>1.20685830625993</b> (−8)	4
3	<b>8.65384735258</b> 162(−2)	10	44	− <b>5.79101041144717</b> (−9)	3
4	− <b>2.55693135993</b> 185(−2)	10	45	<b>3.31772120246795</b> (−10)	2
5	<b>1.12833957531</b> 029(−2)	10	46	<b>2.50396655966956</b> (−9)	4
6	<b>1.91331418311</b> 528(−3)	10	47	− <b>2.63458948729947</b> (−9)	2
7	− <b>7.2975080454</b> 6592(−3)	9	48	<b>1.37788599118725</b> (−9)	2
8	<b>5.6326371615</b> 2084(−3)	11	49	− <b>7.81162737992002</b> (−11)	1
9	− <b>2.3500947967</b> 8666(−3)	10	50	− <b>6.03279317962677</b> (−10)	2
10	− <b>9.8080317234</b> 8266(−4)	11	51	<b>6.81931704576686</b> (−10)	2
11	<b>1.9626662946</b> 7748(−3)	10	52	− <b>3.28515036709205</b> (−10)	2
12	− <b>7.4665149896</b> 5064(−4)	10	53	<b>1.55739979004667</b> (−11)	2
13	<b>6.2342454604</b> 1238(−5)	8	54	<b>1.45343899357852</b> (−10)	2
14	<b>2.6552146868</b> 4286(−4)	10	55	− <b>1.50932163298223</b> (−10)	2
15	− <b>2.7705639389</b> 1716(−4)	9	56	<b>7.77541520065562</b> (−11)	1
16	<b>1.5162845459</b> 3015(−4)	9	57	− <b>3.13825577609254</b> (−12)	0
17	− <b>2.195326394</b> 05031(−5)	8	58	− <b>3.49090836910167</b> (−11)	2
18	− <b>5.638655164</b> 22603(−5)	9	59	<b>3.81299801519005</b> (−11)	1
19	<b>8.0109389549</b> 2351(−5)	7	60	− <b>1.80595862796830</b> (−11)	1
20	− <b>3.793225667</b> 60060(−5)	8	61	<b>5.14543788238336</b> (−13)	0
21	<b>5.7107905999</b> 1865(−6)	7	62	<b>8.14500297893374</b> (−12)	0
22	<b>1.2874849731</b> 4507(−5)	8	63	− <b>8.14981179616867</b> (−12)	0
23	− <b>1.524196328</b> 07077(−5)	7	64	<b>4.01633148267675</b> (−12)	0
24	<b>8.6006099099</b> 2634(−6)	7	65	− <b>7.88269192623829</b> (−16)	0
25	− <b>1.152924962</b> 05150(−6)	7	66	− <b>1.87087975129455</b> (−12)	1
26	− <b>3.173474197</b> 00668(−6)	10	67	<b>1.87619156640006</b> (−12)	1
27	<b>4.084740893</b> 34245(−6)	8	68	− <b>8.09085848892250</b> (−13)	0
28	− <b>1.908030893</b> 22337(−6)	7	69	− <b>2.23213396539257</b> (−14)	0
29	<b>1.572635936</b> 98569(−7)	6	70	<b>3.65055237624289</b> (−13)	0
30	<b>7.659745745</b> 15494(−7)	5	71	− <b>3.22862986722333</b> (−13)	0
31	− <b>8.254934654</b> 02272(−7)	5	72	<b>1.42466073970884</b> (−13)	0
32	<b>4.406783584</b> 91772(−7)	5	73	<b>1.01974866208093</b> (−14)	0
33	− <b>3.75086774</b> 588730(−8)	5	74	− <b>6.70935839889025</b> (−14)	0
34	− <b>1.81333085</b> 487258(−7)	6	75	<b>5.65566193806084</b> (−14)	0
35	<b>2.178209409</b> 24056(−7)	6	76	− <b>1.81107706179081</b> (−14)	0
36	− <b>1.03803895</b> 641095(−7)	5	77	− <b>3.75286265719837</b> (−15)	0
37	<b>7.454660314</b> 90200(−9)	2	78	<b>7.41273630237582</b> (−15)	0
38	<b>4.323537080</b> 53661(−8)	5	79	− <b>3.58842381320799</b> (−15)	0
39	− <b>4.621621089</b> 36192(−8)	4	80	<b>9.27951452130156</b> (−16)	0
40	<b>2.447319975</b> 61589(−8)	3			

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**Table 9**

The leading coefficients for Model B by the CTM as  $D = \frac{5}{11}$  and  $L = 40$ , where “Sig.” denotes the number of significant digits.

<i>i</i>	All digits	Sig.	<i>i</i>	All digits	Sig.
0	<u>4.64855923719</u> 710(−1)	12	41	− <u>1.300023583325</u> 16(−9)	3
1	<u>1.43638755250</u> 735(−1)	12	42	− <u>7.80024521628</u> 120(−9)	3
2	<u>3.03295087132</u> 863(−1)	12	43	<u>9.055611970656</u> 26(−9)	3
3	<u>6.4933539294</u> 9688(−2)	11	44	− <u>4.34532621085</u> 886(−9)	3
4	− <u>1.9185755903</u> 7167(−2)	11	45	<u>2.49039737143</u> 173(−10)	2
5	<u>8.4664172091</u> 7033(−3)	9	46	<u>1.87878617629</u> 899(−9)	4
6	<u>1.4356419362</u> 5392(−3)	11	47	− <u>1.9768823864</u> 4581(−9)	3
7	− <u>5.475634201</u> 98196(−3)	10	48	<u>1.03398435114</u> 461(−9)	2
8	<u>4.2264099603</u> 3476(−3)	11	49	− <u>5.8710651686</u> 8838(−11)	1
9	− <u>1.763377219</u> 59811(−3)	9	50	− <u>4.5263185860</u> 2659(−10)	3
10	− <u>7.359388112</u> 28234(−4)	11	51	<u>5.11713910025</u> 082(−10)	2
11	<u>1.472672948</u> 52949(−3)	9	52	− <u>2.4656413201</u> 4254(−10)	2
12	− <u>5.602447382</u> 37451(−4)	9	53	<u>1.17540988491</u> 699(−11)	2
13	<u>4.6778225460</u> 9653(−5)	7	54	<u>1.09023964290</u> 564(−10)	2
14	<u>1.992321798</u> 42158(−4)	9	55	− <u>1.1326515688</u> 3767(−10)	2
15	− <u>2.078873304</u> 16082(−4)	8	56	<u>5.8391411592</u> 7364(−11)	1
16	<u>1.1377335204</u> 8571(−4)	10	57	− <u>2.4059435847</u> 3458(−12)	0
17	− <u>1.647247829</u> 54602(−5)	8	58	− <u>2.6173699283</u> 8217(−11)	1
18	− <u>4.230925526</u> 06481(−5)	8	59	<u>2.86235616588</u> 793(−11)	1
19	<u>6.010952102</u> 97653(−5)	8	60	− <u>1.3578806652</u> 8575(−11)	2
20	− <u>2.846220391</u> 74530(−5)	8	61	<u>4.13104658171</u> 134(−13)	0
21	<u>4.285051851</u> 33372(−6)	7	62	<u>6.0983027861</u> 2258(−12)	1
22	<u>9.660553594</u> 29563(−6)	8	63	− <u>6.1166869745</u> 2900(−12)	0
23	− <u>1.143670071</u> 52096(−5)	8	64	<u>3.0250903131</u> 3060(−12)	1
24	<u>6.453407584</u> 72969(−6)	8	65	− <u>1.3244335915</u> 0393(−14)	0
25	− <u>8.650891977</u> 98438(−7)	6	66	− <u>1.3983711173</u> 8702(−12)	0
26	− <u>2.381194191</u> 93964(−6)	8	67	<u>1.4102711597</u> 1752(−12)	0
27	<u>3.064956806</u> 08180(−6)	9	68	− <u>6.1306406119</u> 4651(−13)	0
28	− <u>1.431677682</u> 60352(−6)	8	69	− <u>1.1172933231</u> 9077(−14)	0
29	<u>1.180016707</u> 60093(−7)	6	70	<u>2.71166432489</u> 572(−13)	0
30	<u>5.747436680</u> 86085(−7)	6	71	− <u>2.4184291703</u> 7684(−13)	0
31	− <u>6.194032819</u> 60835(−7)	5	72	<u>1.07820342879</u> 944(−13)	0
32	<u>3.306599628</u> 26024(−7)	5	73	<u>6.48150279384</u> 403(−15)	0
33	− <u>2.814440643</u> 84515(−8)	4	74	− <u>4.9800033937</u> 7940(−14)	0
34	− <u>1.360619971</u> 76876(−7)	5	75	<u>4.26088897852</u> 638(−14)	0
35	<u>1.634404374</u> 69177(−7)	5	76	− <u>1.4087034253</u> 9560(−14)	0
36	− <u>7.788858645</u> 30271(−8)	5	77	− <u>2.35170407080</u> 490(−15)	0
37	<u>5.593629265</u> 20973(−9)	4	78	<u>5.3267272603</u> 5767(−15)	0
38	<u>3.2441325580</u> 3418(−8)	6	79	− <u>2.6087218790</u> 6031(−15)	0
39	− <u>3.467805446</u> 48267(−8)	4	80	<u>6.78726134655</u> 898(−16)	0
40	<u>1.8363385716</u> 5517(−8)	4			

**Table 10**

The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = \pi/4$  as  $D = \frac{1}{2}$ .

<i>k</i>	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	1.472968896483033	1.472968896483033	0	Symmetric
2	2.883590172061964	2.8217116472135599	0.5941451264556297	Anti-symmetric
3	4.960770646814888	4.860030994229132	0.9946578031827807	Symmetric
4	6.991301093667654	6.883058973276537	1.225475480258939	Anti-symmetric
5	9.006921531098730	8.898754211636639	1.391692835452230	Symmetric
6	11.01596796529159	10.91026771875466	1.522369376147110	Anti-symmetric

**Table 11**

The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = \pi/4$  as  $D = \frac{5}{11}$ .

<i>k</i>	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	1.516014376393271	1.516014376393271	0	Symmetric
2	2.876843140370182	2.831918346035538	0.5064236721188217	Anti-symmetric
3	4.954286448691338	4.866771089072888	0.9270886593260231	Symmetric
4	6.985315586102505	6.888090199255578	1.161398830878450	Anti-symmetric
5	9.001406678875039	8.902765006809342	1.328945985368559	Symmetric

**Table 12**  
The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = \pi/4$  as  $D = 1$ .

$k$	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	1.0000000000000000	1.0000000000000000	0	Anti-symmetric
2	2.959322163347230	2.739593356324596	1.119024534342417	Symmetric
3	5.026167618453784	4.808250761274224	1.463928121698337	Anti-symmetric
4	7.048671547685778	6.845135158415644	1.681634695817452	Symmetric
5	9.058170509436751	8.868825977288248	1.842383988914041	Anti-symmetric

**Table 13**  
The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = 3\pi/4$  as  $D = \frac{1}{2}$ .

$k$	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	0.6018077748921469	0.6018077748921469	0	Anti-symmetric
2	0.7480105399586175	0.7480105399586175	0	Symmetric
3	1.197155208081384	1.197155208081384	0	Symmetric
4	1.515859334469746	1.515859334469746	0	Anti-symmetric
5	1.769542924220030	1.769542924220030	0	Anti-symmetric
6	2.314991817379920	2.311922516085420	0.1191696024716203	Symmetric
7	2.987516960464614	2.980572754944220	0.2035771144995205	Anti-symmetric
8	3.657905678935235	3.648833242301695	0.2574683006910641	Symmetric
9	4.327132525489046	4.316814106856850	0.2986500627605933	Anti-symmetric
10	4.995656188906412	4.984586568184292	0.3323815609140698	Symmetric
11	5.663722020440595	5.652198969558337	0.3611010015873168	Anti-symmetric
12	6.331473472644984	6.319685412164409	0.3861769362397296	Symmetric
13	6.999000115449453	6.987070716017204	0.4084671656771136	Anti-symmetric
14	7.666360777162631	7.654373422385060	0.4285499694351287	Symmetric
15	8.333595680316281	8.321607690233166	0.4468360021735137	Anti-symmetric
16	9.000733241572054	8.988784537764895	0.4636285361999712	Symmetric
17	9.667794090581419	9.655912680380271	0.4791585194405920	Anti-symmetric

**Table 14**  
The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = 3\pi/4$  as  $D = \frac{5}{11}$ .

$k$	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	0.6073157318928521	0.6073157318928521	0	Anti-symmetric
2	0.7394036691720525	0.7394036691720525	0	Symmetric
3	1.209777500433687	1.209777500433687	0	Symmetric
4	1.491410518278246	1.491410518278246	0	Anti-symmetric
5	1.797134735821410	1.797134735821410	0	Anti-symmetric
6	2.314293111851054	2.313209321200484	0.07081838654211584	Symmetric
7	2.986799444886649	2.981659858658838	0.1751439728844643	Anti-symmetric
8	3.657186684714524	3.649772911248499	0.2327490991816481	Symmetric
9	4.326421108148834	4.317640684137880	0.2754968742348289	Anti-symmetric
10	4.994957136285802	4.985323780447036	0.3100703104809264	Symmetric
11	5.663037791558908	5.652863867215116	0.3393038864471669	Anti-symmetric
12	6.330805206958780	6.320290643545786	0.3647200975597538	Symmetric
13	6.998348187195060	6.987625912969009	0.3872485630966425	Anti-symmetric
14	7.665725112460134	7.654886082677711	0.4075052895718210	Symmetric
15	8.332975944262848	8.322083760631754	0.4259224914142927	Anti-symmetric
16	9.000128951202512	8.989228811532170	0.4428165672076442	Symmetric
17	9.667204684174683	9.656329075638490	0.4584268629864869	Anti-symmetric

The fundamental solutions also satisfy the Cauchy–Navier equation (see [13,40]). However, since they are smooth functions, poor solutions near the concave corners are obtained. For the corners with the clamped and free stress boundary conditions, we may solicit the combined algorithms in [19,20] to choose both fundamental solutions and the leading particular solutions (3.24) and (3.25) with  $v_k$  given in Tables 10, 11, 13 and 14.

**8. Concluding remarks**

To close this paper, let us address the novelties in this paper.

1. The singularity properties are explored for linear elastostatics at corners with the clamped and the free stress boundary conditions. The singularity properties for corner with the *clamped boundary conditions* in this paper are new, compared with the existing literature, such as Williams [38], Lin and Tong [28], Jirousek and Venkstesh [14], Jirousek and Wroblewski [15], Piltner [32], Domingues et al. [6], and Qin [36], where only the free traction conditions of corners are discussed. There also exists lack for a systematic analysis on the singularity properties and particular solutions of linear elastostatics, although many papers have cited and used them. This paper is also devoted to develop such a systematic analysis. By the separation techniques, the singularity properties and particular solutions satisfying the

**Table 15**

The leading powers  $v_k = \alpha_k + i\beta_k$  for  $\Theta = 3\pi/4$  as  $D = 1$ .

$k$	$ v_k $	$\alpha_k$	$\pm \beta_k$	Notes
1	0.5444837367824639	0.5444837367824639	0	Symmetric
2	0.9085291898460988	0.9085291898460988	0	Anti-symmetric
3	1	1	0	Anti-symmetric
4	1.645586951597062	1.629257376759570	0.2312505471151075	Symmetric
5	2.322898854922502	2.301327060714402	0.3158367455250936	Anti-symmetric
6	2.995276274143960	2.971843773159274	0.3739312054164139	Symmetric
7	3.665422571806732	3.641420097721594	0.4187867020567064	Anti-symmetric
8	4.334377325063357	4.310377291541205	0.4554935790867853	Symmetric
9	5.002626455466441	4.978902198949798	0.4866254681226541	Anti-symmetric
10	5.670427007109263	5.647111773660799	0.5136838120241800	Symmetric
11	6.337927206429769	6.315083377631776	0.5376273872874201	Anti-symmetric
12	7.005218173009443	6.982870441506594	0.5591082619629875	Symmetric
13	7.672358758411265	7.650511172997483	0.5785911420085266	Anti-symmetric
14	8.339388500645805	8.318033687816522	0.5964194271257102	Symmetric
15	9.006334830959638	8.985459178447304	0.6128543381700597	Anti-symmetric
16	9.673217297603455	9.652803949157894	0.6280993597962390	Symmetric

complicated boundary conditions can be derived by calculus. See Remark 5.2.

- The explicit singular solutions with respect to the displacements  $u$  and  $v$  are first found for the interior crack (i.e., the crack tip with  $\Theta = \pi$ ). For the free traction boundary conditions of the crack tip, the explicit complex solutions were also provided by Piltner [32] (also see [6]). Since  $z = u + iv$ , the explicit complex solutions are equivalent to the explicit solutions in Sections 5 and 6. However, since the solutions of  $u$  and  $v$  satisfying the Cauchy–Navier equations (2.14) and (2.15) are simpler than the complex solutions in [32], the explicit singular solutions of the crack tip in this paper may find wider applications.
- Two crack models (called Models A and B) are designed, and their highly accurate solutions are obtained by the collocation Trefftz method (CTM). The leading coefficient reaches 12–14 significant digits by the computation in double precision. Those accurate solutions can be used, to examine other numerical solutions for singularity problems as in [19].
- The singular solutions for linear elastostatics at corners are *essential* in both theory and computation. The algorithms to seek the leading powers  $v_k = \alpha_k + i\beta_k$  of the corner solution  $O(r^{v_k})$  are given in Section 7, and in particular, the value of  $v_k$  for  $\Theta = \pi/4$  and  $\Theta = 3\pi/4$ , are provided numerically. Those powers are important for the finite element analysis, and for designing new numerical algorithms. Note that the interior crack, the rectangular corner and the concave corner of the L-shaped domain are most often used in engineering.
- The analysis of corner singularity in this paper is made directly from the Cauchy–Navier equation (2.5), and it is different from the biharmonic equations at corner for the free stress boundary conditions in Williams [38]. Both the approaches and the results in this paper are significant for linear elastostatics with singularity.
- Since the linear elastostatics is more complicated than Poisson's, biharmonic and Helmholtz equations, the analysis and results of this paper may greatly enrich both the linear elastostatics theory and its numerical methods, such as the combined method [19] and the Trefftz methods, which include the boundary approximation method [26,18], the collocation Trefftz method [24,3], the hybrid Trefftz method [10,11,14,15,36], the boundary collocation techniques [16], the Trefftz-FEM [5,37], the dual method [33], etc.

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